

<div>DESIGN CODES AND LOADING: INTERNATIONAL BUILDING CODE (2015 EDITION)</div> <div>LIVE LOADS: ROOF: 20 PSF</div> <div>SNOW LOADS: ROOF SNOW: 40 PSF</div> <div>LATERAL LOADS: WIND: V₅₀ =115 MPH, EXP C, K_{z1} = 1.0 SEISMIC: S_s=0.332, S₁=0.083, S_{ds}= 0.340, S_{d1}= 0.133, I_e=1.0 RISK CATEGORY II SEISMIC SOIL SITE CLASS D SEISMIC DESIGN CATEGORY C</div> <div>MISC LOADS: PARTITION: 5 PSF (OUT OF PLANE)</div> <div>GENERAL: THE INTERNATIONAL BUILDING CODE AND STANDARDS SHALL GOVERN ALL MATERIALS AND WORKMANSHIP.</div> <div>ALL TEMPORARY SHORING OR BRACING IS THE RESPONSIBILITY OF THE CONTRACTOR. THE DRAWINGS REFLECT THE FINAL FINISHED CONDITION OF THE STRUCTURE.</div> <div>THESE DRAWINGS ARE NOT INTENDED TO SHOW EACH AND EVERY CONDITION, BUT INDICATE THE GENERAL CONSTRUCTION WHERE CONDITIONS ARE NOT SPECIFICALLY DETAILED. SIMILAR CONDITIONS SHALL BE USED AT THE DISCRETION AND APPROVAL OF THE ARCHITECT AND ENGINEER.</div> <div>THE CONTRACTOR IS RESPONSIBLE FOR ALL JOB SITE SAFETY AS WELL AS ALL MEANS, METHODS, AND SEQUENCES OF CONSTRUCTION TO SAFELY PERFORM THE WORK. A/E ENGINEERS HAS NO EXPERTISE IN NOR HAS BEEN RETAINED TO PROVIDE REVIEW OF THE CONTRACTORS SAFETY PRECAUTIONS AS THEY RELATE TO THE CONSTRUCTION OF THIS PROJECT.</div> <div>IF ANY ERROR OR OMISSION APPEARS IN THESE DRAWINGS, SPECIFICATIONS, OR OTHER DOCUMENTS, THE CONTRACTOR SHALL NOTIFY THE OWNER IN WRITING OF SUCH OMISSION OR ERROR BEFORE PROCEEDING WITH THE WORK OR ACCEPT FULL RESPONSIBILITY FOR THE COST TO RECTIFY SAME. VERIFY AND CORROBORATE OPENINGS IN FLOORS, WALLS AND ROOF WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.</div> <div>THE ARCHITECTURAL DRAWINGS SHALL BE REFERENCED FOR WALLS, FINISHES AND DIMENSIONS. DIMENSIONS PROVIDED ON THE STRUCTURAL DRAWINGS ARE FOR REFERENCE ONLY AND SHALL BE VERIFIED BY THE ARCHITECTURAL DRAWINGS.</div> <div>DRAWINGS ARE NOT TO BE SCALED.</div> <div>SHOP DRAWINGS: SHOP DRAWINGS ARE SPECIFICALLY REQUIRED FOR THE FOLLOWING ITEMS.<ul style="list-style-type: none">• REINFORCING STEEL• STRUCTURAL STEEL• SUBMIT MIX DESIGN FOR REVIEWTHESE DRAWINGS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER AND ARCHITECT FOR REVIEW. THE REVIEW WILL BE FOR THE DESIGN INTENT ONLY. THE SHOP DRAWINGS SHALL BE SUBMITTED BEFORE PROCEEDING WITH FABRICATION AND SHALL ALLOW TWO WEEKS MINIMUM FOR REVIEW. WE WILL REVIEW ONLY ONE REPRODUCIBLE SET AND ONE COPY TO BE RETAINED BY A/E.</div> <div>DEFERRED SUBMITTALS: DEFERRED SUBMITTALS ARE THOSE PORTIONS OF THE DESIGN WHICH HAVE NOT BEEN PERMITTED UNDER THE BASE BUILDING APPLICATION AND WHICH ARE REQUIRED TO BE SUBMITTED TO THE BUILDING OFFICE FOR APPROVAL. SUBMITTALS SHALL NOT BE INSTALLED PRIOR TO APPROVAL BY THE BUILDING OFFICIAL. ALL SUBMITTALS SHALL BEAR THE STAMP OF A REGISTERED STRUCTURAL WASHINGTON STATE PROFESSIONAL ENGINEER. SUBMITTALS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER OF RECORD PRIOR TO THE SUBMISSION TO BUILDING OFFICIAL. SUBMITTAL WILL BE REVIEWED FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT ONLY. ALL NECESSARY ANCHORAGE, BRACING, TIES AND CONNECTION MATERIAL SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR. THESE ELEMENTS INCLUDE BUT ARE NOT LIMITED TO:<ul style="list-style-type: none">• EXTERIOR ROOF AND WALL CLADDING SYSTEMS INCLUDING CURTAINWALL, PRECAST CONCRETE PANELS, PRE-ENGINEERED BRICK PANELS, VENEER ANCHORAGE SYSTEM• EXTERIOR NON-BEARING COLD FORMED STEEL FRAMING• INTERIOR NON-BEARING COLD FORMED STEEL FRAMING INCLUDING SUSPENDED CEILING AND SOFFITS• STEEL STARS AND RAILINGS• HANDRAILS AND GUARD RAILS• TEMPORARY SHORING SYSTEMS• FLAGPOLES, ANTENNAS, AND SATELLITE DISH• MECHANICAL, ELECTRICAL, PLUMBING, AND SPRINKLER SUPPORT• EQUIPMENT ANCHORAGE</div> <div>FOOTING AND FOUNDATION LOADING: SOILS REPORT: NA ALLOWABLE SOIL BEARING PRESSURE: 1500 PSF (PER EXISTING DRAWINGS) SPECIFIED SOIL SITE CLASS: D ALL SOIL CRITERIA MUST BE VERIFIED IN THE FIELD. ALL DATA LISTED WAS ASSUMED PER INFORMATION PROVIDED. ALL FOOTING DIMENSIONS ARE MINIMUMS. ALL FOOTINGS SHALL REST ON COMPACTED STRUCTURAL FILL OR UNDISTURBED EARTH SUBGRADE WALLS ARE DESIGNED ASSUMING A "DRAINED" CONDITION. SEE CIVIL AND MECHANICAL DRAWINGS FOR SUBGRADE DRAINAGE SYSTEMS. SUBGRADE WALLS SHALL ATTAIN THEIR DESIGN STRENGTH PRIOR TO PLACING BACKFILL. THE CONTRACTOR SHALL PROVIDE TEMPORARY BRACES FOR WALLS IF BACKFILL IS PLACED BEFORE WALLS AND SLABS ACHIEVE THEIR DESIGN STRENGTH OR IF WALL RELIES ON UPPER SLAB FOR SUPPORT.</div> <div>REINFORCED CONCRETE: MINIMUM CONCRETE PROPERTIES: ALL CONCRETE EXPOSURE CATEGORIES PER ACI 318 TABLE 19.3.1.1<ul style="list-style-type: none">• CONCRETE WALLS & FOOTINGS:<ul style="list-style-type: none">- EXPOSURE: F1/F2- 28 DAY STRENGTH, f_c = 4,500 psi- NORMAL WEIGHT CONCRETEINTERIOR SLABS ON GRADE:<ul style="list-style-type: none">- EXPOSURE: F0- 28 DAY STRENGTH, f_c = 3,000 psi- NORMAL WEIGHT CONCRETESLABS ON METAL DECK:<ul style="list-style-type: none">- EXPOSURE: F0- 28 DAY STRENGTH, f_c = 3,000 psi- NORMAL WEIGHT CONCRETEGENERAL REQUIREMENTS: ALL CONCRETE WITH EXPOSURES EXCEEDING F0 SHALL HAVE AIR ENTRAINMENT PER ACI 318 TABLE 19.3.3.1 OR AS NOTED:<ul style="list-style-type: none">- CONCRETE EXPOSED (F1) TO MOISTURE AND FREEZING/THAWING SHALL HAVE 5% AIR ENTRAINMENT +/-1.5%- CONCRETE EXPOSED (F2/F3) TO CONTINUOUS MOISTURE OR SOIL AND FREEZING/THAWING SHALL HAVE 6% AIR ENTRAINMENT +/-1.5%CONCRETE SLUMP SHALL BE 3" MAX (A TARGET SLUMP OF 5" FOR POST TENSIONED SLABS WITH A MAXIMUM OF 6") PRIOR TO ADDITION OF ADMIXTURES AND 6" MAX SLUMP AFTER ADDITION. CALCIUM CHLORIDE MAY NOT BE USED. NOMINAL AGGREGATE SIZE SHALL BE LIMITED TO 3/4" MAXIMUM, U.N.O. OR AS APPROVED BY THE ENGINEER. ALL CONCRETE SHALL HAVE 5 1/2 SACKS MINIMUM OF CEMENT PER CUBIC YARD AND A MAXIMUM WATER/CEMENT RATIO =0.45. FLYASH MAY BE USED TO REPLACE UP TO A MAXIMUM OF 25% OF CEMENT BY WEIGHT. CONCRETE SUPPLIER SHALL COORDINATE EXPECTED WATER VAPOR TRANSMISSION RATES DURING CURING OF THE CONCRETE WITH CONTRACTOR AND ARCHITECT TO ALLOW FOR FLOORING/ROOFING TO BE INSTALLED. FOR ALL AUGER CAST PILING, PROVIDE NOT LESS THAN 9 SACKS OF CEMENT PER CUBIC YARD OF GROUT. ELECTRICAL CONDUIT PLACED IN CONCRETE SHALL BE RIGID STEEL OR FLEXIBLE PLASTIC CONDUIT. NO ALUMINUM IS ALLOWED. FOR CONDUIT PLACED IN SLABS THE MAXIMUM OUTSIDE DIAMETER SHALL NOT EXCEED 1/8 TIMES SLAB THICKNESS AND IT SHALL BE LOCATED IN THE MIDDLE THIRD. NO MORE THAN ONE LAYER OF CONDUIT MAY BE USED. ANCHOR CONDUIT TO A MINIMUM OF #4 @ 12" O.C. PERPENDICULAR TO CONDUIT AND EXTENDING 12" MINIMUM BEYOND. FOR CONDUIT PLACED IN CONCRETE OVER METAL DECKING, CONDUIT MAY RUN IN THE VALLEY OF THE DECK FLUTES ONLY WITH A MINIMUM OF 1" COVER ABOVE AND NO MORE THAN ONE CONDUIT PER DECK FLUTE. CONDUIT SHALL NOT BE LOCATED IN FOOTINGS, BEAMS OR GIRDERS WITHOUT PRIOR APPROVAL OF THE ENGINEER. NON-SHRINK GROUT FOR BASE PLATES SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 6,000 PSI. GROUT SHALL BE AN APPROVED CEMENTITIOUS MATERIAL WITH NATURAL AGGREGATES AND INSTALLED PER THE MANUFACTURERS DIRECTIONS. IF FLOWABLE GROUT MIX IS NOT ADEQUATE FOR COVERAGE AN EPOXY GROUT MAY BE USED. CONCRETE PLACED DURING FREEZING OR NEAR-FREEZING WEATHER SHALL COMPLY WITH REQUIREMENTS OF ACI 318-26.5.4. CONCRETE PLACED DURING HOT WEATHER SHALL COMPLY WITH ACI 318-26.5.5. CONCRETE CLEAR COVER FOR REINFORCING STEEL IS AS FOLLOWS: FOOTINGS AND OTHER UNFORMED SURFACES, EARTH FACE: 3" FORMED SURFACES IN DIRECT CONTACT WITH EARTH OR WEATHER: 2" FOR #6-#18, 1-1/2" FOR #19 TO #5 BAR SLABS, WALLS AND JOISTS NOT EXPOSED TO EARTH OR WEATHER: 1-1/2" FOR #14-#18, 3/4" FOR UP TO #5 BAR BEAMS AND COLUMNS NOT EXPOSED TO EARTH OR WEATHER: 1-1/2"</div> <div>REINFORCING STEEL: GENERAL REQUIREMENTS: STANDARDS FOR ALL REINFORCING STEEL: ASTM A615 GRADE 60. EPOXY COATED REINFORCING STEEL: ASTM A775 GRADE 60. DETAILING (BENDS, HOOKS, SPLAYS, ETC) SHALL BE IN ACCORDANCE WITH ACI 315 AND 318 (LATEST EDITION). MINIMUM LAP AT ALL CONTINUOUS REINFORCEMENT: 30 BAR DIAMETERS OR 1'-6" U.N.O., SEE TABLE FOR REQUIRED LENGTHS CORNER BARS SHALL BE PROVIDED AT ALL CORNERS AND WALL OR FOOTING INTERSECTIONS. CORNER BAR LAPS SHALL BE A MINIMUM OF 30 BAR DIAMETERS OR 1'-6"; SEE TABLE FOR REQUIRED LENGTHS. ALL OPENINGS IN CONCRETE SLABS, WALLS, ETC. MUST HAVE (2) #5 MIN. U.N.O. TRIM BARS ALL AROUND THE OPENING. TRIM BARS MUST EXTEND 2'-6" PAST EVERY CORNER. FIELD BENDING OF BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE IS NOT ALLOWED UNLESS APPROVED BY THE STRUCTURAL ENGINEER. STANDARD FOR ALL WELDED WIRE MESH: ASTM A-186. LAP ALL EDGES 1 1/2 MESH MINIMUM. ALL REINFORCING STEEL SHALL BE FREE OF GREASE, OIL, OR MUD. ETC. AT THE TIME OF CONCRETE PLACEMENT. INTENDED COATINGS SHALL BE PER PLANS OR APPROVED BY ENGINEER. ALL BOLTS SHALL BE TIED IN PLACE SO THAT THEY ARE HELD IN THE CORRECT FINAL POSITION THROUGHOUT THE CONCRETE PLACEMENT, DO NOT "WET STICK" BOLTS. WELDING REQUIREMENTS: STANDARDS FOR WELDINGS OF REINFORCING BARS: ANSI/AWS D1.4-86. REINFORCEMENT SHALL NOT BE WELDED UNTIL A CHEMICAL ANALYSIS SUFFICIENT TO DETERMINE THE CARBON EQUIVALENT IS PERFORMED. THE CARBON EQUIVALENT OF REINFORCING STEEL BARS SHALL BE CALCULATED FROM THE CHEMICAL COMPOSITION AS SHOWN IN THE MILL TEST REPORT. IF MILL TEST REPORTS ARE NOT AVAILABLE, A CHEMICAL ANALYSIS SHALL BE MADE OF BARS REPRESENTATIVE OF THE BARS TO BE WELDED. THE CARBON EQUIVALENT (C.E.) SHALL NOT EXCEED .55. REINFORCEMENTS MEETING ASTM A706 DEEMED TO MEET THE WELDABILITY REQUIREMENTS MAY BE USED IN LIEU OF TESTING. SPECIAL INSPECTION IS REQUIRED FOR ALL FIELD WELDING.</div>	<div>STRUCTURAL STEEL: GENERAL REQUIREMENTS:<ul style="list-style-type: none">• W & WT SHAPES: ASTM A992 (F_y=50 ksi)• SQUARE OR RECTANGULAR STRUCTURAL TUBE (HSS): ASTM A500 GRADE C (F_y = 50 ksi)• ROUND STRUCTURAL TUBE (HSS): ASTM A500 GRADE C (F_y = 46 ksi)• PIPE MEMBERS: ASTM A53 GRADE B (F_y = 35 ksi)• PLATES, CHANNELS, ANGLES, & RODS: ASTM A36 (F_y = 36 ksi)• PLATES (NOTED AS GRADE 50 ON DRAWINGS): ASTM A572 (F_y = 50 ksi)• ALL STAINLESS STEEL: TYPE 316L (F_y = 30 ksi)• CONNECTION BOLTS: ASTM F1554 (GRADE A325-N/C)• INSTALL ALL CONNECTION BOLTS PER CRITERIA FOR SLIP CRITICAL CONNECTIONS W/ CLASS A FAYING SURFACES, U.N.O.• ANCHOR BOLTS OR ANCHORS RODS: ASTM F1554 (GRADE 30) (F_y = 35 ksi)• FURNISH ANCHOR RODS WITH MATCHING DOUBLE HEAVY HEX NUTS AT THE END EMBEDDED IN CONCRETECAST-IN HEADED BOLTS SHALL BE PLACED ACCURATELY INTO FINAL POSITION PRIOR TO POURING CONCRETE. ADDING BOLTS AFTER A POUR OR "WET STICKING" IS NOT ALLOWED. PROVIDE WASHERS FOR ALL BOLTS AS REQUIRED BY AISC. AS A MINIMUM PROVIDE STANDARD CUT WASHERS UNDER ALL NUTS. ONE COAT OF APPROVED SHOP PAINT MINIMUM TO ALL STEEL NOT EMBEDDED IN CONCRETE. FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC (ASD) SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS. STEEL NOTED AS GALVANIZED SHALL BE HOT-DIP GALVANIZED UNO. CONTRACTOR SHALL DESIGN AND SUPPLY ALL ADDITIONAL MISCELLANEOUS METALS THAT ARE INDICATED IN THE ARCHITECTURAL DRAWINGS OR THOSE METALS WHICH ARE FOUND TO BE NECESSARY TO SUPPORT THE ARCHITECTURAL FINISHES OR OTHER BUILDING SYSTEMS. ALL FRAMING AND CONNECTIONS DESIGNED BY THE CONTRACTOR SHALL NOT RESULT IN ECCENTRIC LOADS BEING APPLIED TO THE PRIMARY STRUCTURE OR LATERAL LOADS BEING APPLIED TO THE BOTTOM FLANGE OF STEEL BEAMS. SUBMIT CALCULATIONS STAMPED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF THE PROJECT ALONG WITH SHOP DRAWINGS NOTING THE LOADING IMPOSED ON THE PRIMARY STRUCTURE. WELDING REQUIREMENTS ALL WELDING SHALL MEET AWS CODES FOR ARC WELDING IN BUILDING CONSTRUCTION. EXCEPT AS SPECIFIED IN THE SPECIAL PROCEDURE OF THIS SECTION, ELECTRODES MAY BE E70 (70 KSI MINIMUM), ELECTRODES MUST BE KEPT DRY AT ALL TIMES. MINIMUM WELDS SHALL BE 3/16" OR AS NOTED IN SECTION 1.2b OF AISC (WHICHEVER IS LARGER) ALL WELDING OF STAINLESS STEEL SHALL USE E309 ELECTRODES WITH A GMAc PROCESS FIELD AND SHOP WELDING SHALL BE PERFORMED BY AWS CERTIFIED WELDERS.</div> <div>POST INSTALLED ANCHORS: MASONRY INSTALLATION: U.N.O. INSTALLATION SHALL BE IN GROUTED CELLS ONLY. IF EMBEDMENT MUST BE PROVIDED IN AN UN-GROUTED CELL, NEW GROUT SHALL BE ADDED AT CUT END EXTENDING TO THE HORIZONTAL BOND BEAM BELOW. ONE EMBEDDED ITEM ONLY PER GROUTED CELL IS ALLOWED AND NO EMBEDDED ITEMS ARE ALLOWED WITHIN 8" OF A FREE EDGE. (INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARD INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY AN APPLICABLE CERTIFICATION PROGRAM.) MANUFACTURERS RECOMMENDATIONS AND ICBO/ACI EVALUATION REPORT SHALL BE FOLLOWED DURING THE PREPARATION AND INSTALLATION OF ALL GROUTED BOLTS, RODS AND REINFORCING BARS. NO REINFORCING SHALL BE CUT TO INSTALL ANCHORS. DEFECTIVE OR ABANDONED HOLES SHALL BE FILLED WITH NON-SHRINK GROUT THAT AS A MINIMUM MATCHES THE ADJACENT CONCRETE/MASONRY GROUT STRENGTH.</div> <div>ADHESIVE MASONRY ANCHORS:<ul style="list-style-type: none">- GROUTED MASONRY: DEWALT AC108+ GOLD (ICC ESR-3200, ESR-3200 LABO) OR APPROVED EQUAL- UN-GROUTED MASONRY: DEWALT AC108+ GOLD W/ SCREEN TUBE (ICC ESR-3200, ESR-3200 LABO) OR APPROVED EQUAL- UNREINFORCED MASONRY: DEWALT AC108+ GOLD W/ SCREEN TUBE (ICC ESR-4105, ESR-4105 LABO) OR APPROVED EQUALMECHANICAL MASONRY ANCHORS:<ul style="list-style-type: none">- GROUTED MASONRY: DEWALT POWER-STUD+ SD1 (ICC ESR-2966, ESR-2966 LABO) OR APPROVED EQUAL- GROUTED MASONRY: DEWALT SCREW-BOLT+ (ICC ESR-4042, ESR-4042 LABO) OR APPROVED EQUALADHESIVE CONCRETE ANCHORS:<ul style="list-style-type: none">- DEWALT PURET110+ (ICC ESR-3298, ESR-3298 LABO) OR APPROVED EQUALMECHANICAL CONCRETE ANCHORS:<ul style="list-style-type: none">- DEWALT POWER-STUD+ S02 (ICC ESR-2902, ESR-2902 LABO) OR APPROVED EQUAL- DEWALT SCREW-BOLT+ (ICC ESR-3889, ESR-3889 LABO) OR APPROVED EQUALWHERE SPECIAL INSPECTIONS ARE REQUIRED PER THE ANCHOR (ICBO/ACI) EVALUATION REPORT, INSPECTIONS SHOULD BE AS FOLLOWS: 1) FOR ALL ANCHORS PRIOR TO CONCEALMENT, VERIFY: ANCHOR TYPE; ANCHOR DIMENSIONS; ANCHOR SPACING AND EDGE DISTANCE 2) FOR EACH ANCHOR TYPE AND SIZE, THE INSPECTOR SHALL BE ONSITE TO CONTINUOUSLY INSPECT A MINIMUM OF (10) ANCHORS INSTALLED BY EACH INSTALLER FOR CONFORMANCE WITH THE APPLICABLE ICBO/ACI EVALUATION REPORT. IF THE TOTAL NUMBER OF ANCHORS INSTALLED BY EACH INSTALLER IS LESS THAN 10, THEN ALL ANCHORS INSTALLED BY EACH INSTALLER SHALL BE INSPECTED FOR CONFORMANCE WITH THE APPLICABLE ICBO/ACI EVALUATION REPORT. PROVIDED ALL ANCHORS ARE INSTALLED CORRECTLY PER MANUFACTURER'S INSTRUCTIONS, PROVIDE PERIODIC INSPECTION ON A MINIMUM OF 10 PERCENT OF THE NEXT 1000 ANCHORS BY EACH INSTALLER AND A MINIMUM OF 5 PERCENT OF THE REMAINING ANCHORS BY EACH INSTALLER. INSPECTIONS SHOULD OCCUR ONE A WEEK AT A RANDOM TIME WHILE ANCHOR INSTALLATION IS ONGOING. ANY NON-COMPLIANCE ISSUES SHALL RESET THE INSPECTION REQUIREMENTS TO THE BEGINNING OF THIS SECTION. ALL NON-COMPLAINT ANCHORS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD FOR REVIEW AND SHALL BE BROUGHT INTO COMPLIANCE EITHER THRU TESTING OR REINSTALLATION. 3) FREQUENCY OF TESTING BEYOND THOSE LISTED IN ITEM 2 SHALL BE PER THE (ICBO/ACI) EVALUATION REPORT REQUIREMENTS) (SECTION 1901.3.4 OR 1910.4.5 OF THE CBC) 4) THE SPECIAL INSPECTOR SHALL PROVIDE DOCUMENTATION AT THE END OF ANCHOR INSTALLATIONS STATING THE MINIMUM NUMBER OF ANCHORS WERE INSPECTED.</div> <div>POWDER ACTUATED FASTENERS: MANUFACTURERS RECOMMENDATIONS AND ICC REPORTS SHALL BE FOLLOWED DURING THE PREPARATIONS AND INSTALLATION OF ALL PAF'S. ALL REINFORCEMENT IN CONCRETE MUST BE LOCATED PRIOR TO INSTALLATION TO PREVENT DAMAGE DURING INSTALLATION. WASHERS ARE REQUIRED WHEN PAF'S ARE USED WITH WOOD MEMBERS. HLTI X-U (POWDER ACTUATED FASTENER): WHERE X-U PAF'S ARE SPECIFIED PER PLAN 0.157" DIAM. HLTI X-U #4 PAF'S (ESR-2269, LARR 25675) SHALL BE USED OR APPROVED EQUAL. AT CONCRETE SUBSTRATES, X-U PAF'S SHALL BE INSTALLED IN THE FOLLOWING MANNER UNO: 1-1/4" MINIMUM PAF EMBEDMENT, 4" MINIMUM SPACING BETWEEN PAF'S, 3" MINIMUM CONCRETE EDGE DISTANCE. MINIMUM CONCRETE SUBSTRATE THICKNESS EQUAL TO 3X THE EMBEDMENT DEPTH OF THE FASTENER. AT CONCRETE METAL DECK SUBSTRATES, X-U PAF'S SHALL BE INSTALLED AS SPECIFIED PER PLAN. AT STRUCTURAL STEEL SUBSTRATES, X-U PAF'S SHALL BE INSTALLED IN THE FOLLOWING MANNER UNO: AT STEEL SUBSTRATES WITH A THICKNESS LESS THAN 1/2" PROVIDE FULL EMBED OF PAF TO STEEL, POINT OF FASTENER TO PENETRATE THROUGH STEEL BASE MATERIAL; AT STEEL SUBSTRATES WITH A THICKNESS GREATER THAN OR EQUAL TO 1/2" PROVIDE MIN 1/2" POINT PENETRATION OF PAF TO STEEL. 1" MINIMUM SPACING BETWEEN PAF'S. 1/2" MINIMUM STEEL EDGE DISTANCE. HLTI X-P (POWER ACTUATED FASTENER): WHERE X-P PAF'S ARE SPECIFIED PER PLAN 0.118" DIAM. HLTI X-P #3 G3 (GAS DRIVEN) OR 0.118" DIAM. HLTI X-P #3 B3 (ELECTRI-MECHANICALLY DRIVEN) PAF'S (ESR-1752, LARR 26662) SHALL BE USED OR APPROVED EQUAL. AT CONCRETE SUBSTRATES, X-P PAF'S SHALL BE INSTALLED IN THE FOLLOWING MANNER UNO: 5/8" PAF EMBEDMENT, 4" MINIMUM SPACING BETWEEN PAF'S, 3" MINIMUM CONCRETE EDGE DISTANCE. MINIMUM CONCRETE SUBSTRATE THICKNESS EQUAL TO 3X THE EMBEDMENT DEPTH OF THE FASTENER. AT CONCRETE METAL DECK SUBSTRATES, X-P PAF'S SHALL BE INSTALLED AS SPECIFIED PER PLAN. AT STRUCTURAL STEEL SUBSTRATES, X-P PAF'S SHALL BE INSTALLED IN THE FOLLOWING MANNER UNO: AT STEEL SUBSTRATES WITH A THICKNESS LESS THAN OR EQUAL TO 3/8" PROVIDE FULL EMBED. OF PAF TO STEEL (POINT OF FASTENER TO PENETRATE THROUGH STEEL BASE MATERIAL); AT STEEL SUBSTRATES WITH A THICKNESS GREATER THAN 3/8" X-P PAF'S ARE NOT PERMITTED FOR INSTALLATION. 1" MINIMUM SPACING BETWEEN PAF'S. 1/2" MINIMUM STEEL EDGE DISTANCE.</div>	<div>REINFORCED CMU MASONRY: CMU PROPERTIES: MASONRY COMPRESSIVE STRENGTH F_m 2,500 PSI (AT 28 DAYS). CMU SHALL BE MEDIUM WEIGHT AND SHALL CONFORM TO ASTM C90, TYPE 1. TYPE 'S' MORTAR (ASTM C-270) HAVING MIN. COMPRESSIVE STRENGTH OF 1,800 PSI AT 28 DAYS IS REQUIRED. RUNNING BOND IS REQUIRED U.N.O. GENERAL REQUIREMENTS: VERIFICATION OF COMPRESSIVE STRENGTH OF MASONRY IS REQUIRED BY PRISM TESTING PRIOR TO AND DURING CONSTRUCTION AT LEVEL " B / C " QUALITY ASSURANCE PER TMS 402/602-13. ALL CELLS ARE TO BE GROUTED SOLID, U.N.O. IF SPECIFIED, "PARTIAL GROUTING" SPECIFIES THAT ONLY CELLS WITH REINFORCEMENT ARE TO BE GROUTED. FILL CELLS WITH GROUT (ASTM C-476) HAVING MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI. IN LIFTS NOT EXCEEDING 5'-4" IN HEIGHT. REINFORCING STEEL SHALL BE DEFORMED BARS PER ASTM A615, GRADE 60 (f_y=60,000 psi). ALL REINFORCEMENT SHALL BE IN PLACE PRIOR TO GROUTING WITH VERTICAL BARS HELD AT TOP, BOTTOM AND 19" DIAMETERS MAX. ON CENTERS. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING FOR MASONRY WALLS. AS REQUIRED, UNTIL CONNECTIONS TO FLOOR AND/OR ROOF DIAPHRAGMS ARE COMPLETED, MINIMAL WALL HAS REACHED DESIGN STRENGTH. MINIMUM VERTICAL STEEL SHALL BE #5 @ 32" O.C. PROVIDE TWO #5 FULL HEIGHT EACH SIDE OF ALL OPENINGS AND AT DISCONTINUOUS ENDS. PROVIDE HORIZONTAL BOND BEAMS WITH TWO #5 AT SPACING INDICATED ON PLANS BUT NOT EXCEEDING 32" O.C. PROVIDE BOND BEAMS AT ALL FLOOR & ROOF LINES AND AT THE TOP OF THE WALL. ALSO PROVIDE A 16" DEEP BOND BEAM WITH TWO #5 HORIZONTALLY (BOTTOM) OVER ALL WALL OPENINGS (EXTEND BOND BEAMS 2'-0" PAST THE OPENING ON EACH SIDE AT PARTIAL GROUTED WALLS. BOND BEAMS SHALL BE CONTINUOUS IN SOLID GROUTED WALLS. ALL HORIZONTAL BARS SHALL BE PLACED IN BOND BEAM OR LINTEL BEAM UNITS. PROVIDE CORNER BARS TO MATCH THE WALL HORIZONTAL REINFORCEMENT AT ALL INTERSECTIONS. ALL EMBEDDED ITEMS SHALL BE GROUTED SOLID IN CELLS. SEE THE SCHEDULE FOR REQUIRED DEVELOPMENT AND LAP LENGTHS FOR ALL REINFORCING STEEL. COLD WEATHER CONSTRUCTION PROVISIONS SHALL BE IMPLEMENTED WHEN AMBIENT TEMPERATURE FALLS BELOW 40 DEGREES FAHRENHEIT PER TMS 402/602-13. HOT WEATHER CONSTRUCTION PER TMS 402/602-13 SHALL BE IMPLEMENTED WHEN AMBIENT AIR TEMPERATURE EXCEEDS 100 DEGREES FAHRENHEIT OR 90 DEGREES WITH WINDS OVER 5 MPH. CONDUITS, PIPES AND SLEEVES SHALL NOT BE PLACED IN MASONRY CONSTRUCTION UNLESS SPECIFICALLY APPROVED BY ENGINEER. IF EMBEDMENT IS APPROVED THE CONDUIT, PIPE OR SLEEVE SHALL BE COMPATIBLE WITH THE MASONRY (ALUMINUM NOT ALLOWED), SHALL BE NO CLOSER THAN THREE DIAMETERS (OF THE LARGEST DIAMETER MEMBER) ON CENTER, SHALL BE A MINIMUM OF 1-1/2" CLEAR OF ANY REINFORCING AND SHALL NOT DISPLACE MORE THAN 2% OF THE NET CELL CROSS SECTION. PIPES SHALL NOT CONTAIN LIQUIDS, GAS OR VAPORS AT TEMPERATURE OVER 150 DEGREES, BE SUBJECT TO FREEZING OR AT PRESSURES OVER 55 PSI.</div> <div>ROOF DECK AND WELDING: ROOF DECK 1 1/2" DEEP 20 GAUGE X 36" WIDE. ALL DECKS TO BE CONTINUED OVER AT LEAST THREE SPANS UNLESS OTHERWISE SO INDICATED ON THE DRAWINGS. PROVIDE (4) 5/8" DIA. PUDDLE WELDS PER 36" WIDTH OF DECK OVER EACH SUPPORT. PROVIDE 1 1/2" LONG TOP SEAM WELDS @ 24" O.C. PROVIDE 5/8" DIA. MARGINAL PUDDLE WELDS @ 12" O.C. USE E60 SERIES ELECTRODES FOR DECK WELDING.</div> <div>STEEL STUDS AND JOISTS: GENERAL REQUIREMENTS: STEEL STUD MANUFACTURERS ASSOCIATION, LATEST EDITION SHALL GOVERN FOR ALL STEEL STUDS & JOISTS SPECIFIED. DEPTH, GAUGE, AND SECTION PROPERTIES OF STEEL STUDS AND JOISTS SHALL MEET OR EXCEED THOSE OF THE SECTIONS SPECIFIED. SEE ICC ESR-3064P FOR ADDITIONAL MATERIAL INFORMATION. G-60 GALVANIZED COATING PER ASTM A653 IS REQUIRED FOR ALL STEEL STUDS AND JOISTS ALL SCREWS SHALL BE SELF-TAPPING / SELF-DRILLING FASTENERS THAT ARE ZINC COATED AS MANUFACTURED BY HILTI KWIK-FLEX (ICC ESR-2196, LARR 25095) OR APPROVED EQUAL. 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ALL LINTELS SHALL BE HOT DIP GALVANIZED U.N.O ALL LINTEL ANGLES SHALL BEAR ON A MINIMUM OF 4" OF MASONRY EACH END LINTEL ANGLES SHALL BEAR ON A MINIMUM OF 4" OF MASONRY EACH END PROVIDE VERT. EXPANSION JOINTS IN CONTINUOUS VENEER @ 25' O.C. MAX. TYPICAL. SUPPORTING STRUCTURE TO BE DESIGNED FOR A MINIMUM OUT OF PLANE AND IN PLANE DEFLECTION OF L/600.</div> <div>WHERE VENEER HEIGHTS EXCEED 30 FEET FROM FOUNDATION OR IN SEISMIC DESIGN CATEGORY E OR F VENEER SHALL BE SUPPORTED AT EACH BUILDING LEVEL (DOES NOT INCLUDE ROOF) PER DETAILS ON THE DRAWINGS.</div> <div>MASONRY VENEER TO CMU SUBSTRATE: ADJUSTABLE WALL ANCHORTIES, SHEET METAL SLOTTED ANCHOR SCREWED TO STEEL SUBSTRATE AND WIRE TIE; GALV. FINISH. FURNISH LENGTH OF TIES AS RECOMMENDED BY MANUFACTURER FOR PARTICULAR CONDITIONS OF INSTALLATION IN EACH CASE. MASONRY VENEER TO STEEL OR WOOD SUBSTRATE: ADJUSTABLE WALL ANCHORTIES, PNTLE AND EYE TYPE; GALV. FINISH. FURNISH LENGTH OF TIES AS RECOMMENDED BY MANUFACTURER FOR PARTICULAR CONDITIONS OF INSTALLATION IN EACH CASE. TIES SHALL HAVE A LIP OR HOOK FOR POSITIVE ENGAGEMENT WITH HORIZONTAL VENEER REINFORCEMENT. ANCHOR TO WOOD FRAMING WITH CORROSION RESISTANT 80 COMMON NAIL OR SCREW WITH EQUIVALENT WITHDRAWAL VALUE. ANCHOR TO STEEL FRAMING WITH CORROSION RESISTANT SCREWS THAT HAVE A MINIMUM SHANK DIAMETER OF 0.19 INCH. FOR CORRUGATED SHEET METAL ANCHORS LOCATE FASTENER WITHIN 1/2 INCH OF BEND.</div>	<div>ROOF DECK AND WELDING: ROOF DECK 1 1/2" DEEP 20 GAUGE X 36" WIDE. ALL DECKS TO BE CONTINUED OVER AT LEAST THREE SPANS UNLESS OTHERWISE SO INDICATED ON THE DRAWINGS. PROVIDE (4) 5/8" DIA. PUDDLE WELDS PER 36" WIDTH OF DECK OVER EACH SUPPORT. PROVIDE 1 1/2" LONG TOP SEAM WELDS @ 24" O.C. PROVIDE 5/8" DIA. MARGINAL PUDDLE WELDS @ 12" O.C. 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SPECIAL INSPECTIONS:

GENERAL REQUIREMENTS

ALL SPECIAL INSPECTIONS SHALL MEET THE REQUIREMENTS OF THE 2015 IBC, CHAPTER 17

ALL INSPECTIONS AS REQUIRED BY SECTION 110 OF THE 2015 INTERNATIONAL BUILDING CODE ARE REQUIRED. INSPECTIONS SPECIFIED IN THESE NOTES ARE IN ADDITION TO THESE INSPECTIONS.

CITY INSPECTION IS NOT A SUBSTITUTE FOR SPECIAL INSPECTION.

ANY WORK WHICH HAS BEEN COVERED BUT NOT PROPERLY INSPECTED BY THE SPECIAL INSPECTOR AND/OR THE CITY INSPECTOR IS SUBJECT TO REMOVAL OF EXPOSURE.

WHERE SPECIFICALLY REQUIRED, CONTINUOUS INSPECTION IS REQUIRED DURING THE PERFORMANCE OF THE WORK. THIS MAY BE A REQUIREMENT OF THE BUILDING CODE / LOCAL JURISDICTION OR THE MANUFACTURER.

THE SPECIAL INSPECTOR MUST BE CERTIFIED TO PERFORM THE TYPES OF INSPECTION SPECIFIED AND SHALL DEMONSTRATE COMPETENCE TO THE SATISFACTION OF THE BUILDING OFFICIAL.

THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING AND INFORMING THE SPECIAL INSPECTOR OR CITY INSPECTOR AT LEAST ONE WORKING DAY BEFORE THE WORK IS TO BE PERFORMED UNLESS OTHER CONDITIONS ARE AGREED UPON.

REQUIREMENTS OF THE SPECIAL INSPECTOR:

THE SPECIAL INSPECTOR MUST WORK UNDER THE SUPERVISION OF A WASHINGTON LICENSED CIVIL ENGINEER.

THE SPECIAL INSPECTOR MUST PERSONALLY BE FAMILIAR WITH THE DRAWINGS AND MUST PERSONALLY OBSERVE ALL OF THE WORK REPORTED ON

THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING DEPARTMENT AND ENGINEER. ANY DISCREPANCIES SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION; THEN, IF NOT CORRECTED, TO THE BUILDING DEPARTMENT AND ENGINEER.

THE FINAL REPORT SHALL BE SIGNED BY A WASHINGTON LICENSED CIVIL ENGINEER AND SHALL STATE THAT THE WORK WAS IN CONFORMANCE WITH THE APPROVED DRAWINGS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF IBC.

SPECIFIC SPECIAL INSPECTIONS REQUIRED:

- ALL CONCRETE WHERE THE COMPRESSIVE STRENGTH (f'_c) IS GREATER THAN 2,500 PSI EXCEPT AT SLAB ON GRADES.
- ALL PLACEMENT OF ALL BOLTS, REINFORCING, EMBEDS, OR POST TENSION CABLES IN CONCRETE.
- ALL SHEATHED DIAPHRAGMS INCLUDING SHEARS WALLS, FLOORS, AND ROOFS.
- ALL EPOXY GROUTING OF BOLTS OR REINFORCING BARS.
- ALL POST-INSTALLED ANCHORS (SEE "POST-INSTALLED ANCHORS" SECTION FOR TEST RESULTS)
- ALL REINFORCED MASONRY CONSTRUCTION REQUIRES LEVEL "B" QUALITY ASSURANCE INSPECTIONS PER TMS 402/602-13.
- ALL WELDING OF STEEL EXCEPT WELDING PERFORMED IN AN AISC APPROVED SHOP.
- THAT SOIL EXCAVATIONS EXTEND TO DEPTH AND BEARING STRATA.

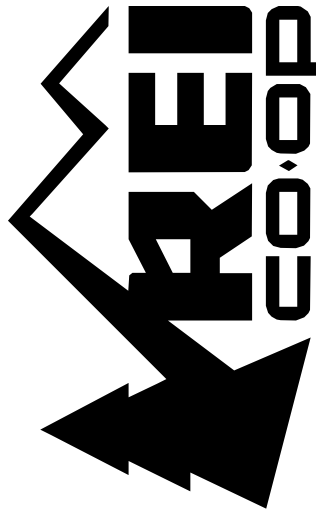
DISCLAIMER:

THESE DOCUMENTS AND THE DESIGN ARE SPECIFIC TO THIS PROJECT ONLY AND MAY NOT BE REUSED IN ANY WAY WITHOUT WRITTEN APPROVAL OF AUE. IT IS OUR INTENT THAT THIS DESIGN MEETS THE NORMAL STANDARD OF CARE WITHIN THIS INDUSTRY. NO OTHER WARRANTY IS PROVIDED OR IMPLIED.

CLIENT STORE NUMBER

XXX

CLIENT INFORMATION



ARCHITECT INFORMATION

CALLISONTKL™

CallisonRTKL Inc.
CallisonRTKL Inc.
U.S. Bank Centre
1420 5th Ave Suite

YYY-YYYYYY YY

CONSULTANT INFORMATION



13228 NE 20th STREET, Ste. 100
BELLEVUE, WA 98004
425 614-0949 / INFO@AU-ENG.CO
JOB NO: 21173

PROJECT INFORMATION

REI-GLENWOOD SPRINGS

3216 S. GLEN AVENUE
GLENWOOD SPRINGS,
CO 81601

SIGNATURE/SEAL

DRAWING ISSUANCE LOG

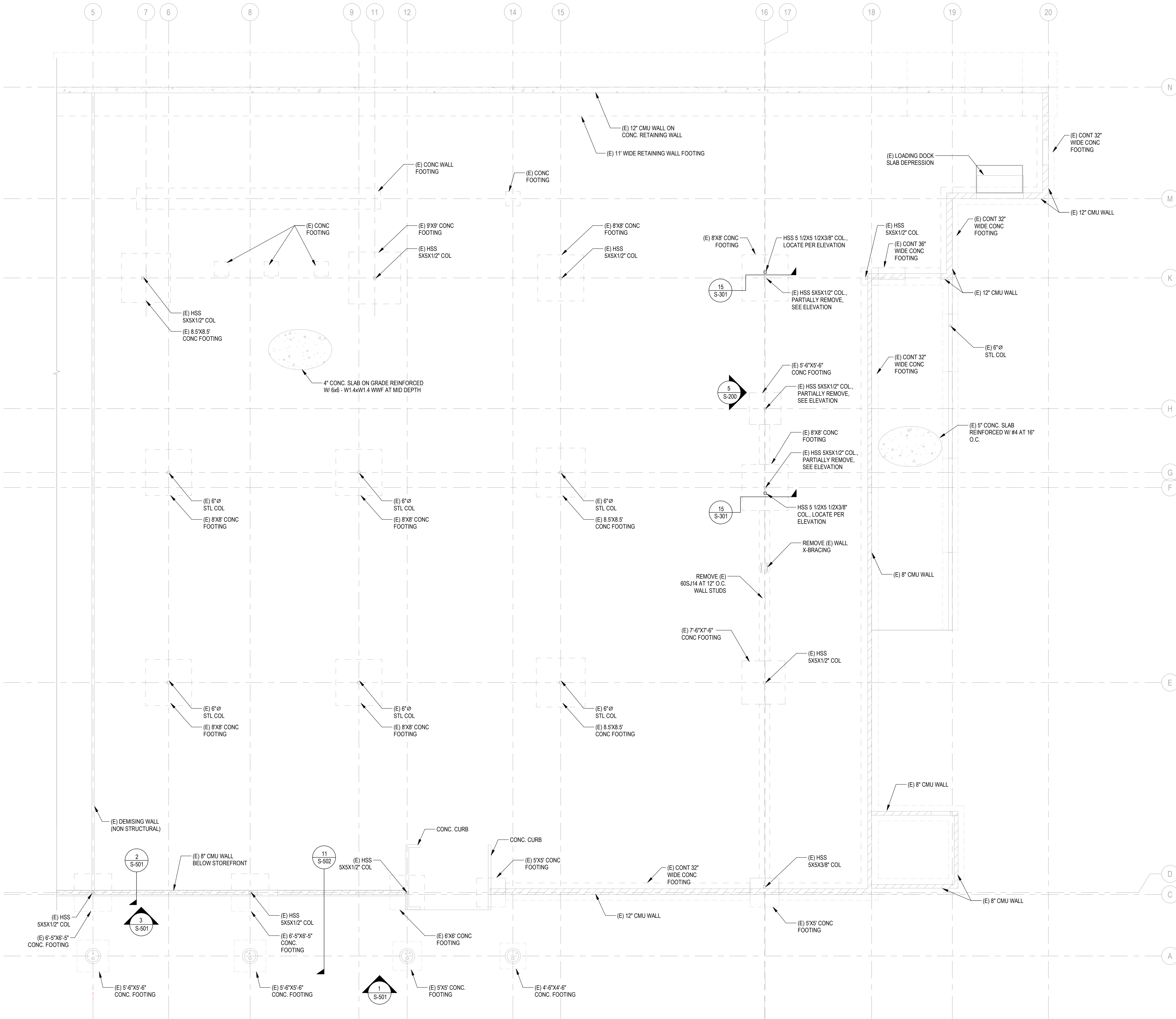
REV	DATE	DESCRIPTION
2	10/21/21	OWNER REVIEW SE

SHEET TITLE

GENERAL NOTES (CONT.)

SHEET NUMBER

S-002



LEVEL 1 - STRUCTURAL PLAN
SCALE: 1/8" = 1'-0"
PLAN NORTH

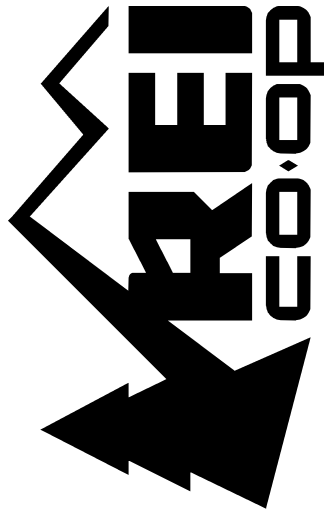
- PLAN NOTES:**
- (E) NON STRUCTURAL INTERIOR PARTITION WALLS NOT SHOWN, UNO.
 - SEE DET. 1S-301 FOR REINFORCEMENT BENDING SCHEDULE
 - SEE DET. 3S-301 FOR TYP. ANCHOR BOLTS
 - SEE DET. 4S-301 FOR TYP. BEAM STRUT/PHOOP ASSEMBLY
 - SEE DET. 5S-301 FOR TYP. SLAB ON GRADE JOINTS
 - SEE DET. 6S-301 FOR TYP. COLUMN BLOCKOUT
 - SEE DET. 8S-301 FOR TRENCH AT (E) SLAB ON GRADE
 - SEE DET. 13S-301 FOR REINF. DEVELOPMENT & SPLICE SCHEDULE
 - SEE DET. 15S-301 FOR TYP. INTERIOR COLUMN TO SPREAD FOOTING CONN.

NOTE:
SPECIAL INSPECTION APPLIES TO THIS PROJECT PER THE GENERAL NOTES. IF THE SPECIAL INSPECTIONS LISTED ARE NOT PROVIDED, TESTING OR REWORK WILL BE REQUIRED FOR FINAL SIGN OFF.


NOTE:
ALL DIMENSIONS SHOWN ON THESE STRUCTURAL PLANS ARE FOR GENERAL REFERENCE ONLY. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND ANY DISCREPANCIES MUST BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER IMMEDIATELY.

NOTE:
THESE DRAWINGS ARE BASED ON A LIMITED AMOUNT OF INFORMATION FOR AN EXISTING BUILDING. CONTRACTOR MUST FIELD VERIFY CONDITIONS AND NOTIFY ARCHITECT AND ENGINEER IF CONDITIONS ARE OTHER THAN AS SHOWN PRIOR TO MODIFICATION.

CLIENT STORE NUMBER
235

CLIENT INFORMATION


ARCHITECT INFORMATION
CALLISONRTKL
CallisonRTKL Inc.
CallisonRTKL Inc.
1402 8th Ave Suite
2400 Seattle, WA 98101
006-132864.81

CONSULTANT INFORMATION

13228 NE 20th STREET, Ste. 100
BELLEVUE, WA 98004
425 614-0949 / INFO@AU-ENG.COM
JOB NO: 21173

PROJECT INFORMATION
REI-GLENWOOD SPRINGS
3216 S. GLEN AVENUE
GLENWOOD SPRINGS,
CO. 81601

SIGNATURES:

REV	DATE	DESCRIPTION
11/05/21	PERMIT SET	
11/08/21	BID SET	

DRAWING SOURCE LOG

REV	DATE	DESCRIPTION
11/05/21	PERMIT SET	
11/08/21	BID SET	

SHEET TITLE
LEVEL 1 - STRUCTURAL PLAN

SHEET NUMBER
S-111



LEVEL 1 - FLOOR PLAN
SCALE: 1/8" = 1'-0"
PLAN NORTH

NOTE:
SPECIAL INSPECTION APPLIES TO THIS PROJECT PER THE GENERAL NOTES. IF THE SPECIAL INSPECTIONS LISTED ARE NOT PROVIDED, TESTING OR REWORK WILL BE REQUIRED FOR FINAL SIGN OFF.

PLAN NOTES:

- SEE SHEETS S-001 FOR STRUCTURAL GENERAL NOTES, ABBREVIATIONS AND REQUIRED SPECIAL INSPECTIONS.
- CONCRETE CURB DETAIL 9/S-301 LOCATION PER ARCH.
- INTERIOR WALL PARTITION KEY:

FH	XX	FULL HT. PARTITION WALL, DETAIL 6/S-601
PH	XX	PARTIAL HT. WALL, DETAIL 8/S-601
PHBB	XX	PARTIAL HT. WALL BOX BEAM BRACED, DETAIL 16/S-602
PHS	XX	PARTIAL HT. STAND-ALONE WALL, DETAIL 19/S-602
OPNG	XX	WALL OPENINGS, DETAIL 12/S-601

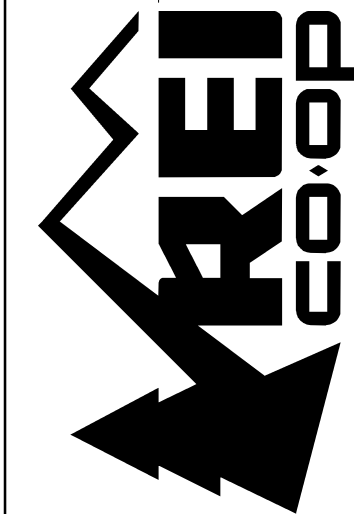
NOTE:
ALL DIMENSIONS SHOWN ON THESE STRUCTURAL PLANS ARE FOR GENERAL REFERENCE ONLY. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND ANY DISCREPANCIES MUST BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER IMMEDIATELY.

NOTE:
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CLIENT STORE NUMBER

235

CLIENT INFORMATION



ARCHITECT INFORMATION

CALLISONRTKL™

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CallisonRTKL Inc.
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006-132564.81

CONSULTANT INFORMATION



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JOB NO: 21173

PROJECT INFORMATION

REI-GLENWOOD SPRINGS

3216 S. GLEN AVENUE
GLENWOOD SPRINGS,
CO. 81601

SIGNATURE/SEAL

DRAWING SOURCE LOG

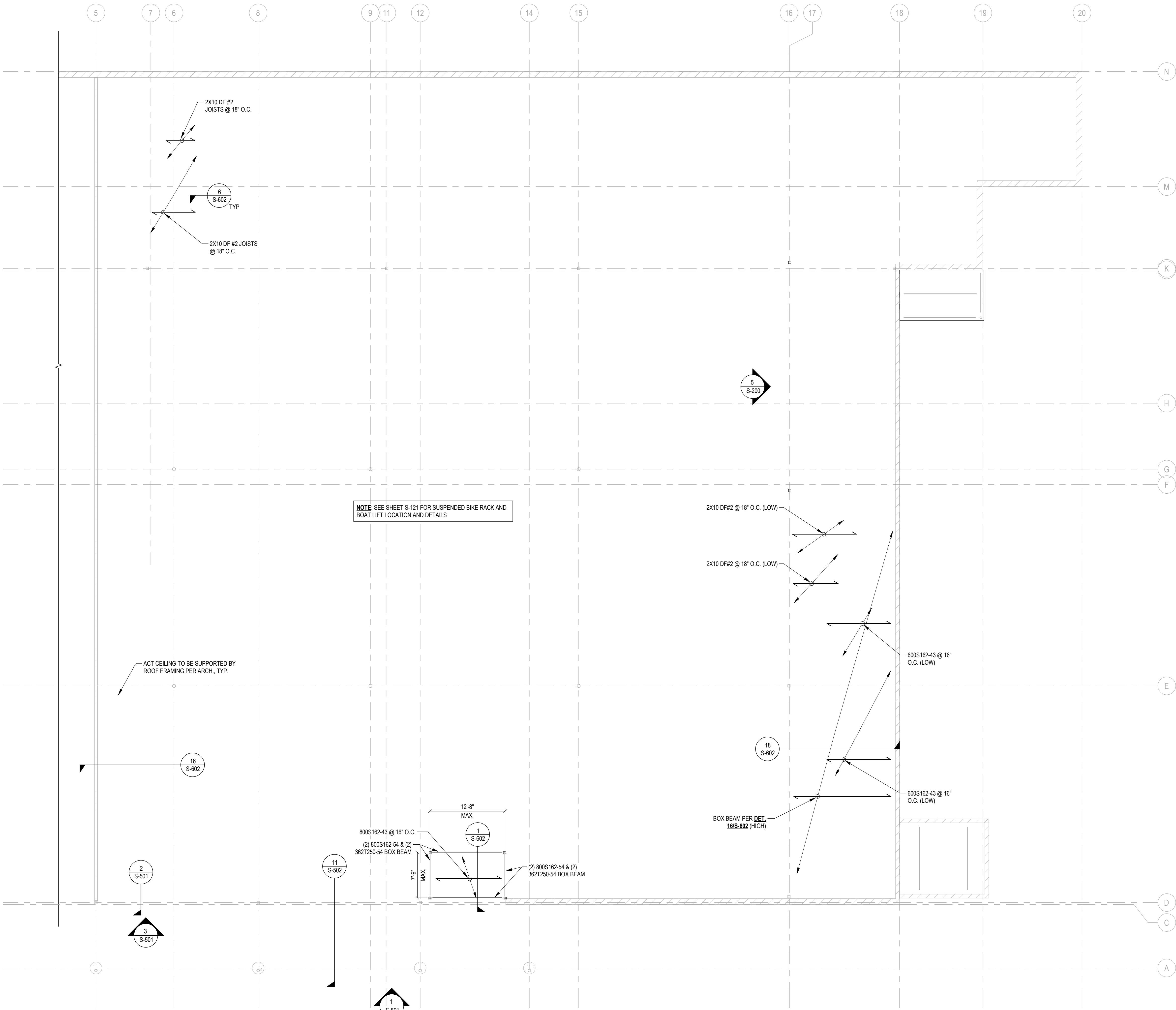
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11/05/21	PERMIT SET	
11/08/21	BID SET	

SHEET TITLE

LEVEL 1 - FLOOR PLAN

SHEET NUMBER

S-112



REFLECTED CEILING PLAN
SCALE: 1/8" = 1'-0"
PLAN NORTH

NOTE:
SPECIAL INSPECTION APPLIES TO THIS PROJECT PER THE GENERAL NOTES. IF THE SPECIAL INSPECTIONS LISTED ARE NOT PROVIDED, TESTING OR REWORK WILL BE REQUIRED FOR FINAL SIGN OFF.

NOTE:
ALL DIMENSIONS SHOWN ON THESE STRUCTURAL PLANS ARE FOR GENERAL REFERENCE ONLY. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND ANY DISCREPANCIES MUST BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER IMMEDIATELY.

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CLIENT STORE NUMBER
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CLIENT INFORMATION

ARCHITECT INFORMATION
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006-132864.81

CONSULTANT INFORMATION

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JOB NO: 21173

PROJECT INFORMATION
REI-GLENWOOD SPRINGS
3216 S GLEN AVENUE
GLENWOOD SPRINGS,
CO. 81601

SIGNATURE/SEAL

DRAWING SOURCE LOG

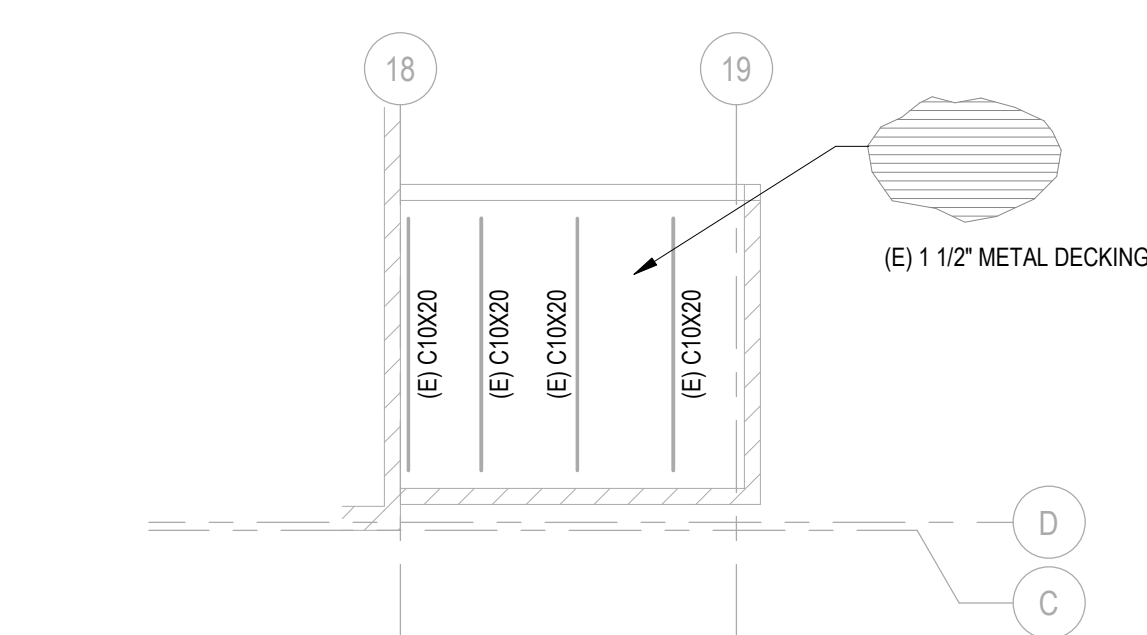
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11/08/21	BID SET	

SHEET TITLE
REFLECTED CEILING PLAN

SHEET NUMBER
S-113



NOTE:
THESE DRAWINGS ARE BASED ON A LIMITED AMOUNT OF
INFORMATION FOR AN EXISTING BUILDING. CONTRACTOR
MUST FIELD VERIFY CONDITIONS AND NOTIFY ARCHITECT
AND ENGINEER IF CONDITIONS ARE OTHER THAN AS
SHOWN PRIOR TO MODIFICATION.



PUMP ROOM ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"

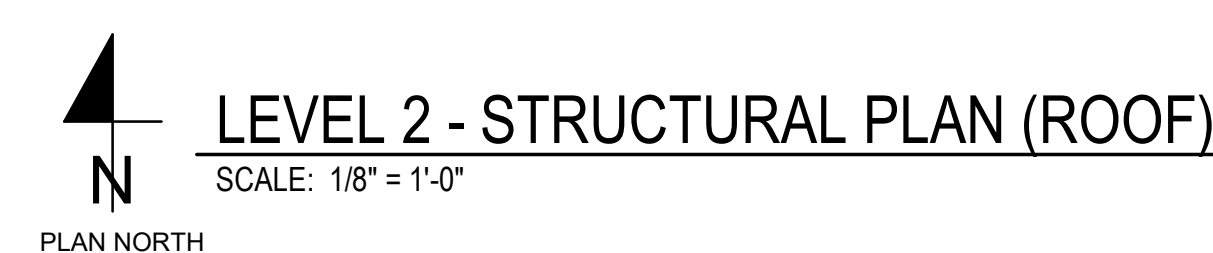
REI-GLENWOOD SPRINGS

IV	DATE	DESCRIPTION
	11/05/21	PERMIT SET
	11/08/21	BID SET

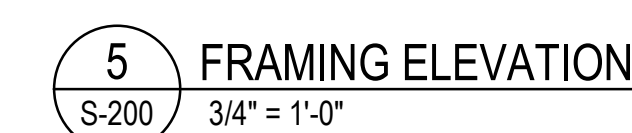
MEZZANINE & LOW ROOF
PLAN

SET NUMBER:

S-115

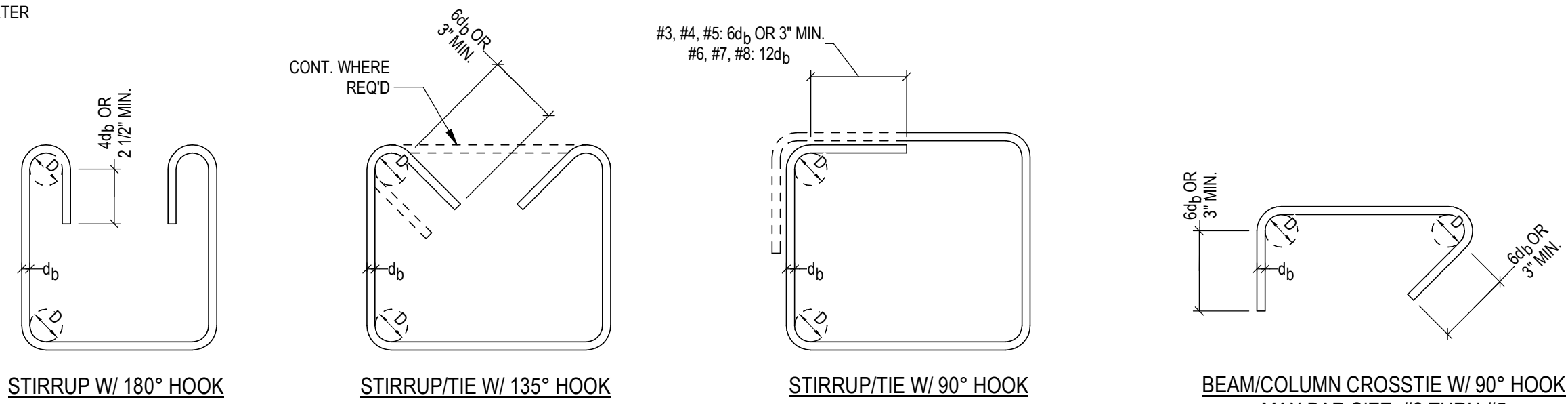


NOTE:
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MUST FIELD VERIFY CONDITIONS AND NOTIFY ARCHITECT
AND ENGINEER IF CONDITIONS ARE OTHER THAN AS
SHOWN PRIOR TO MODIFICATION.



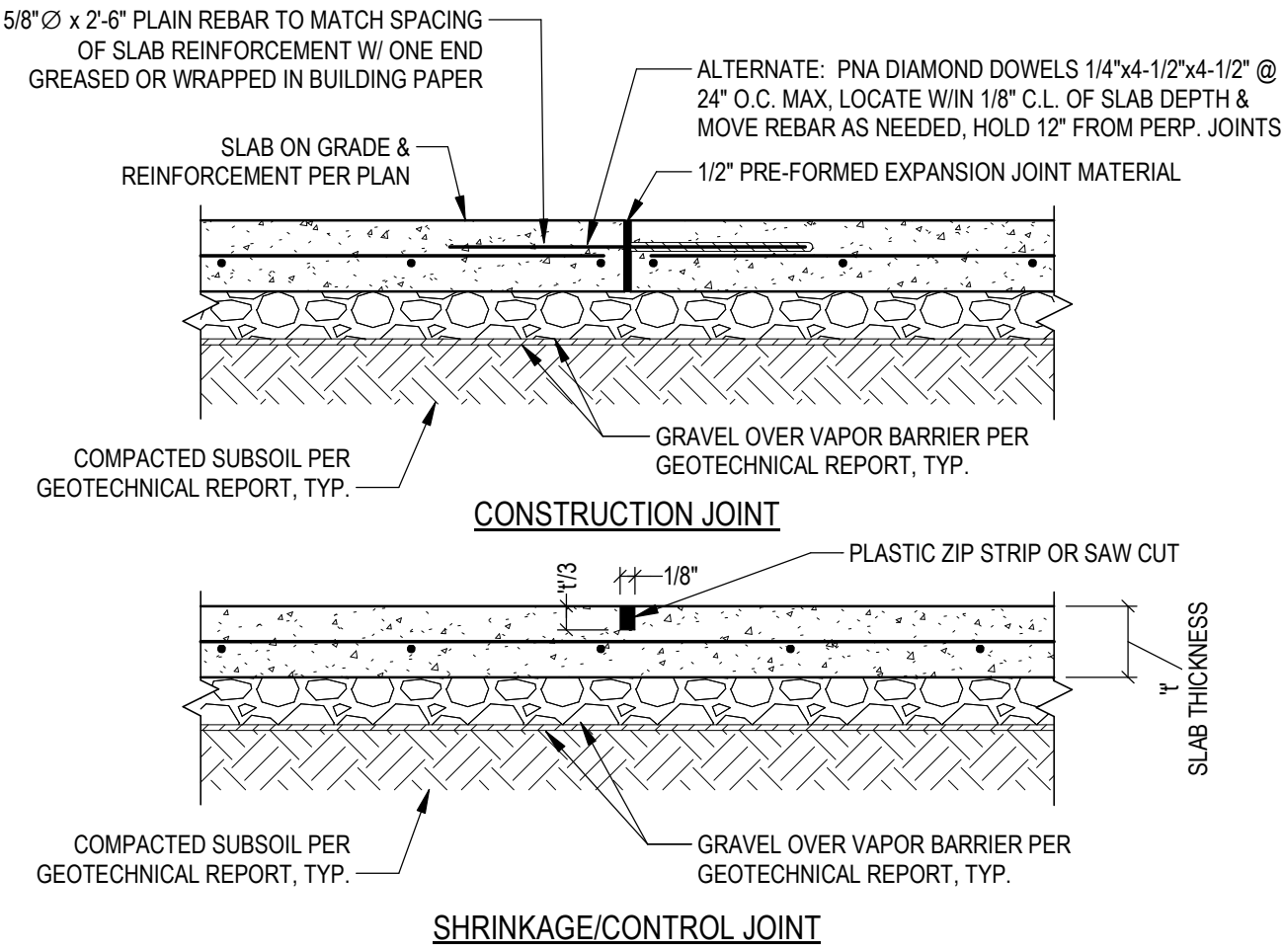
REBAR BENDING SCHEDULE		
BAR SIZES	STANDARD HOOKS	STIRRUP / TIE HOOKS
#3, #4, #5	6d	4d _b
#6, #7, #8	6d	6d _b
#9, #10, #11	8d _b	N/A
#14, #18	10d _b	N/A

d' = INSIDE BEND DIAMETER
d_b = BAR DIAMETER



1 REINFORCEMENT BENDING SCHEDULE

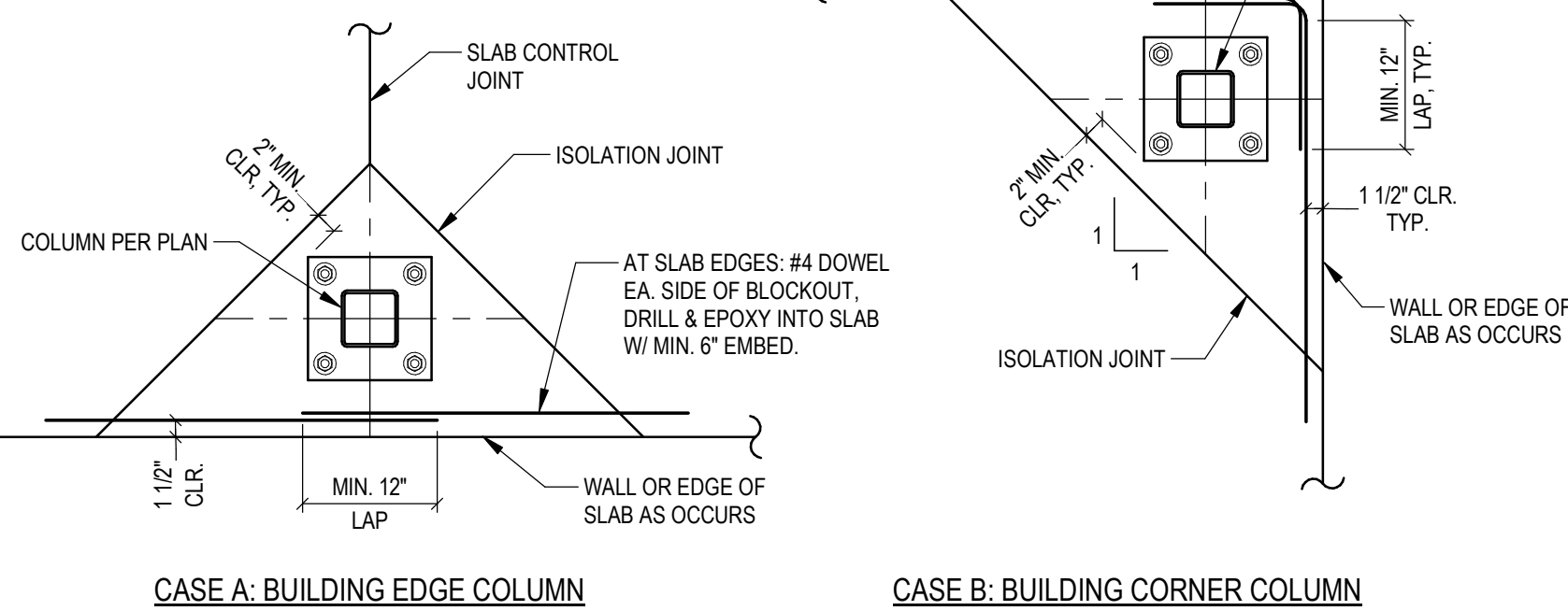
NTS



5 TYPICAL SLAB ON GRADE JOINTS

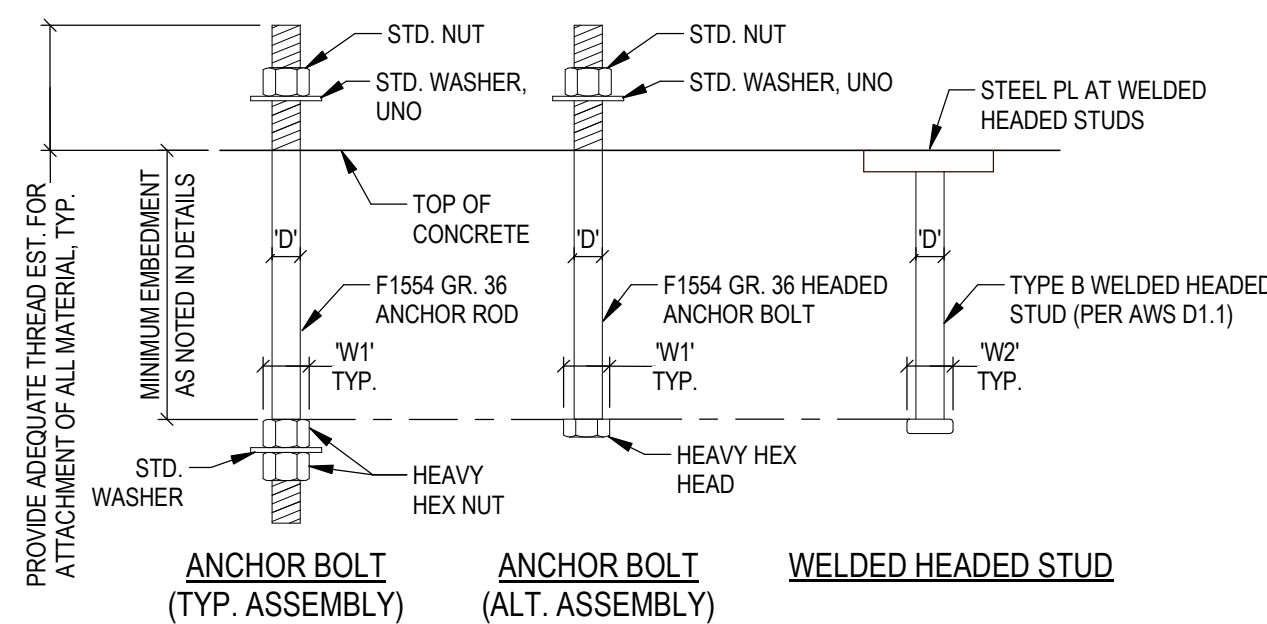
NTS

NOTES:
1. CONCRETE IN BOX OUT SECTIONS SHALL BE POURED AT LEAST 24 HOURS AFTER SLAB AND AFTER COLUMN DEAD LOADS HAVE BEEN APPLIED.



6 TYPICAL COLUMN BLOCKOUT

NTS

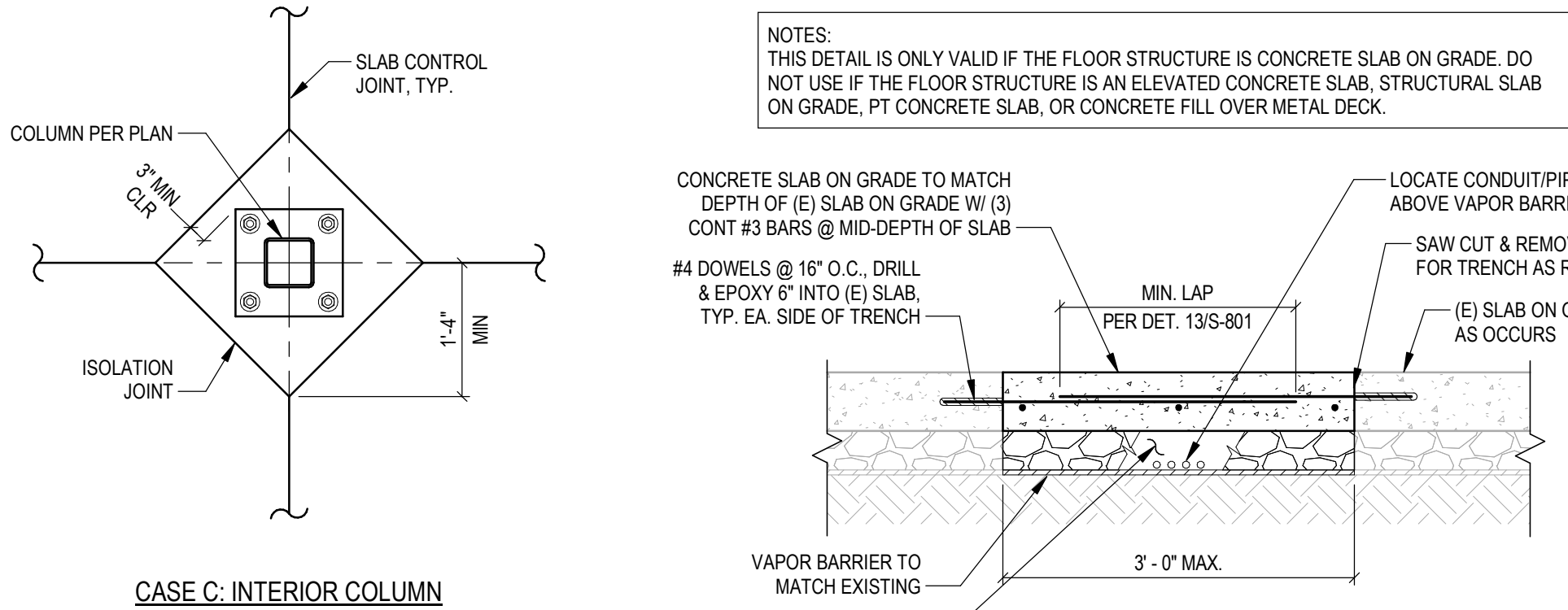


ANCHOR BOLT DIMENSIONS		
BOLT DIAMETER 'D'	MIN. NUT/HEAD DIAMETER 'W1'	MIN. HEAD DIAMETER 'W2'
1/2"	7/8"	1"
5/8"	1-1/16"	1-1/4"
3/4"	1-1/4"	1-1/4"
7/8"	1-7/16"	1-3/8"
1"	1-5/8"	1-5/8"
1-1/8"	1-13/16"	N/A

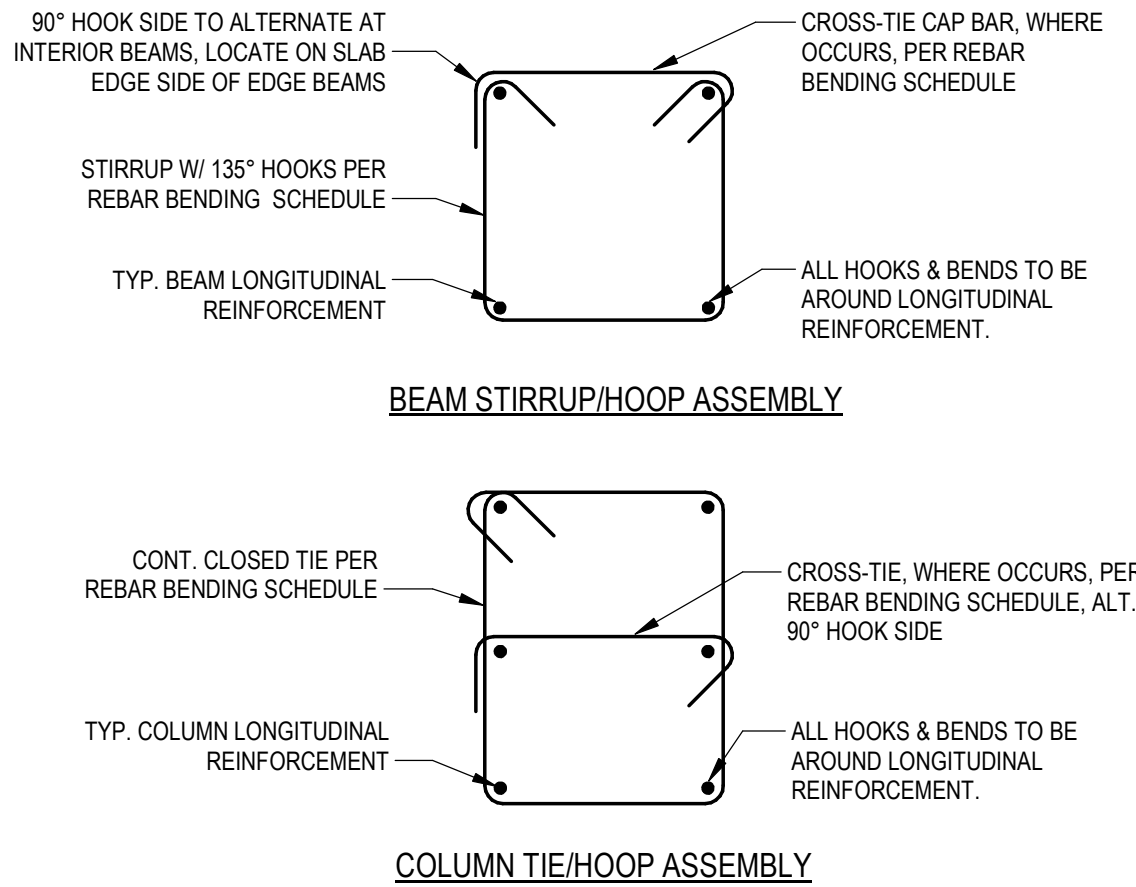
NOTES:
1. EMBEDMENT DISTANCE DOES NOT INCLUDE HEIGHT OF CURBS.
2. PROVIDE 3" OF COVER AT BOTTOM AND SIDES OF ANCHOR BOLTS.

3 TYPICAL ANCHOR BOLTS

NTS

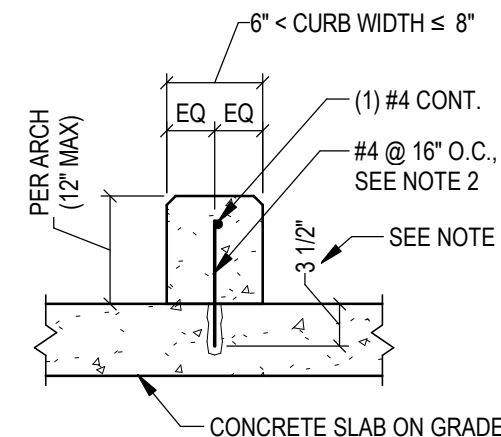


NOTES:
1. SEE DETAIL 13/S-301 FOR REINFORCEMENT BENDING SCHEDULE.



4 TYPICAL BEAM STIRRUP/HOOP COLUMN TIE/HOOP ASSEMBLY

3/4" = 1'-0"



9 CURBS ON SLAB ON GRADE

3/4" = 1'-0"

TYPICAL STRAIGHT (L _d) AND HOOKED (L _{dh}) REINFORCEMENT DEVELOPMENT LENGTH SCHEDULE											
		BAR SIZE:	#3	#4	#5	#6	#7	#8	#9	#10	#11
		BAR DIAMETER, d _b	d _b = 0.375 in	d _b = 0.500 in	d _b = 0.625 in	d _b = 0.625 in	d _b = 0.875 in	d _b = 1.000 in	d _b = 1.128 in	d _b = 1.270 in	d _b = 1.410 in
DEVELOPMENT TYPE	CATEGORY	DESCRIPTION	N.W. CONCRETE f _c (psi)	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER
STRAIGHT	1	MIN. COVER ≥ 1 1/2" MIN. CLEAR SPACING ≥ 3"	3,000	13"	12"	18"	14"	22"	17"	26"	20"
			4,500	12"	12"	14"	12"	18"	14"	21"	17"
			5,000	12"	12"	14"	12"	17"	13"	20"	16"
			6,000	12"	12"	13"	12"	16"	12"	19"	14"
	2	MIN. COVER ≥ 3/4" MIN. CLEAR SPACING ≥ 2"	3,000	13"	12"	22"	17"	32"	25"	43"	33"
			4,500	12"	12"	18"	14"	26"	20"	35"	27"
			5,000	12"	12"	17"	13"	25"	19"	34"	26"
			6,000	12"	12"	16"	12"	23"	18"	31"	24"
HOOKED	STANDARD	ALL OTHERS	3,000	9"	9"	11"	14"	17"	20"	22"	28"
			4,500	7"	7"	9"	12"	14"	16"	18"	21"
			5,000	7"	7"	9"	11"	13"	15"	17"	20"
			6,000	6"	6"	8"	10"	12"	14"	16"	20"

TYPICAL REINFORCEMENT LAP SPICE (L _{st}) DEVELOPMENT LENGTH SCHEDULE											
		BAR SIZE:	#3	#4	#5	#6	#7	#8	#9	#10	#11
		BAR DIAMETER, d _b	d _b = 0.375 in	d _b = 0.500 in	d _b = 0.625 in	d _b = 0.625 in	d _b = 0.875 in	d _b = 1.000 in	d _b = 1.128 in	d _b = 1.270 in	d _b = 1.410 in
LAP CLASS	CATEGORY	DESCRIPTION	N.W. CONCRETE f _c (psi)	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER
CLASS B	1	MIN. COVER ≥ 1 1/2" MIN. CLEAR SPACING ≥ 3"	3,000	17"	16"	24"	19"	29"	23"	34"	26"
			4,500	16"	16"	19"	16"	24"	19"	28"	23"
			5,000	16"	16"	19"	16"	23"	17"	26"	21"
			6,000	12"	12"	16"	12"	23"	18"	31"	24"
	2	MIN. COVER ≥ 3/4" MIN. CLEAR SPACING ≥ 2"	3,000	17"	16"	29"	23"	42"	33"	56"	43"
			4,500	16"	16"	24"	19"	34"	26"	46"	36"
			5,000	16"	16"	23"	17"	33"	25"	45"	34"
			6,000	16"	16"	21"	16"	30"	24"	41"	32"

GENERAL NOTES:
1. ALL DEVELOPMENT AND LAP SPICE LENGTHS SPECIFIED IN SCHEDULES ARE FOR REINFORCING STEEL WITH A F_y = 60,000 psi.
2. *TOP* - HORIZONTAL BARS PLACED SUCH THAT MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE BAR.
3. *OTHER* - ALL VERTICAL BARS AND HORIZONTAL BARS THAT DO NOT MEET THE *TOP* BAR DESIGNATION.
4. UNLESS NOTED OTHERWISE, ALL HOOKED BARS SHALL EXTEND TO THE FAR FACE W/ MIN COVER BEYOND PER NOTE 5.2.
5. ALL HOOKED BAR LENGTHS SPECIFIED IN SCHEDULE ARE FOR REINFORCING WITH:
5.1 SIDE COVER ≥ 2 1/2",
5.2 COVER BEYOND ≥ 2"

LAP SPICE NOTES:
1. ALL SPLICES SHALL BE WIRED IN CONTACT AND STACKED VERTICALLY.
2. SMALLER BAR LAP LENGTH SHALL BE USED WHEN SPLICING DIFFERENT SIZED BARS.
3. LAP LENGTHS SPECIFICALLY DETAILED IN DRAWINGS SHALL GOVERN IN LIEU OF LAP LENGTHS SCHEDULED.

ADJUSTMENTS TO SPECIFIED STRAIGHT DEVELOPMENT AND LAP SPICE LENGTHS:
1. LIGHTWEIGHT CONCRETE: MULTIPLY THE LENGTHS SPECIFIED IN THE SCHEDULE BY 1.33
2. EPOXY-COATED REINFORCEMENT:
2.1. *TOP* BARS: MULTIPLY THE LENGTHS SPECIFIED IN THE SCHEDULE BY 1.3
2.2. *OTHER* BARS: MULTIPLY THE LENGTHS SPECIFIED IN THE SCHEDULE BY 1.5
3. COMBINATIONS OF EFFECTS DUE TO CONCRETE WEIGHT AND EPOXY COATING ARE CUMULATIVE.
L_d / L_{dh} SHALL BE MULTIPLIED BY EACH FACTOR TO FIN THE CORRECT VALUE

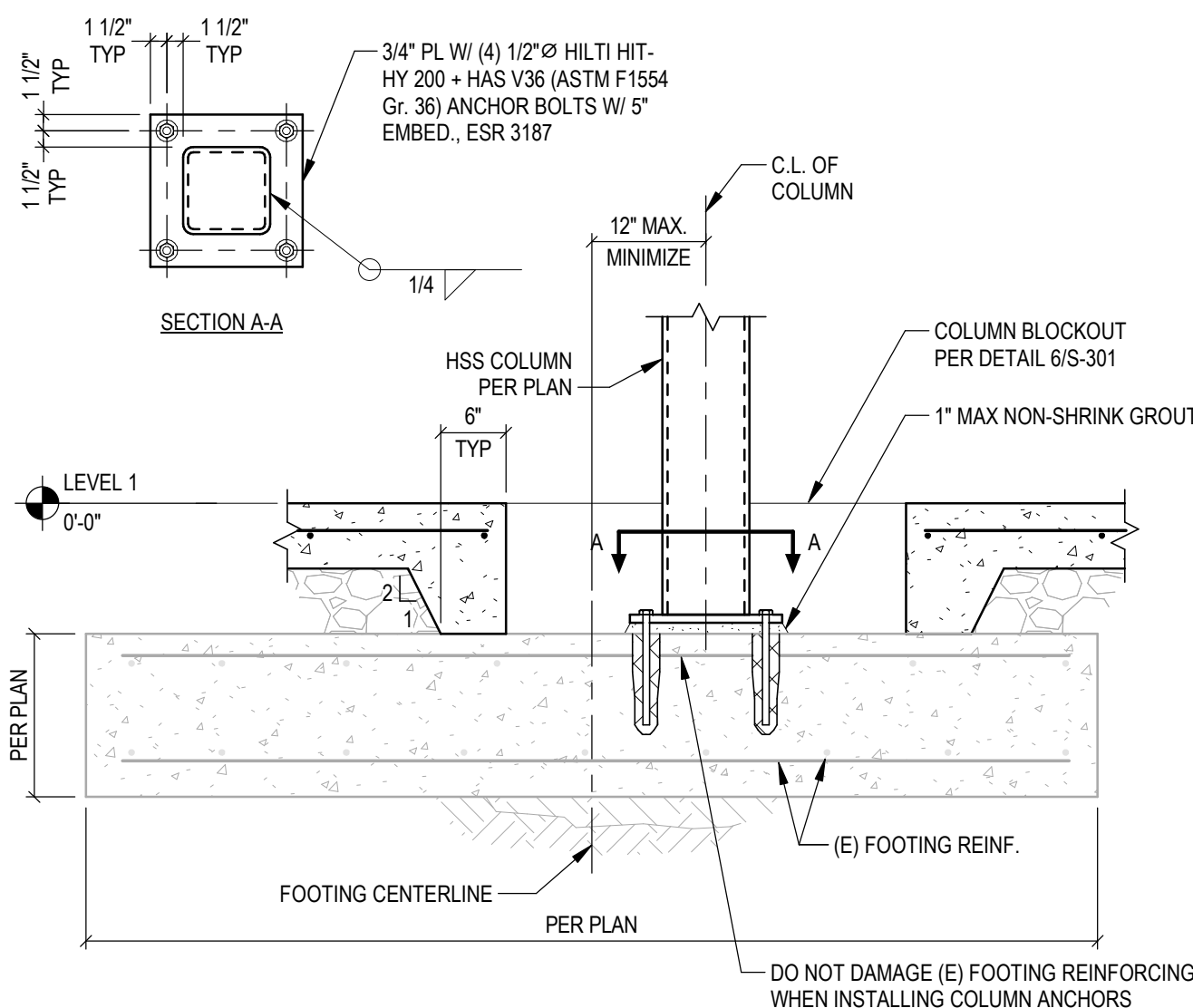
ADJUSTMENTS TO SPECIFIED HOOK DEVELOPMENT LENGTHS:
1. LIGHTWEIGHT CONCRETE: MULTIPLY THE LENGTHS SPECIFIED IN THE SCHEDULE BY 1.33
2. EPOXY-COATED REINFORCEMENT: MULTIPLY THE LENGTHS SPECIFIED IN THE SCHEDULE BY 1.2
3. SIDE COVER < 2 1/2": MULTIPLY THE LENGTHS SPECIFIED IN THE SCHEDULE BY 0.7

13 REINFORCEMENT DEVELOPMENT & SPICE SCHEDULE

NTS

15 TYP. INTERIOR SPREAD FTG

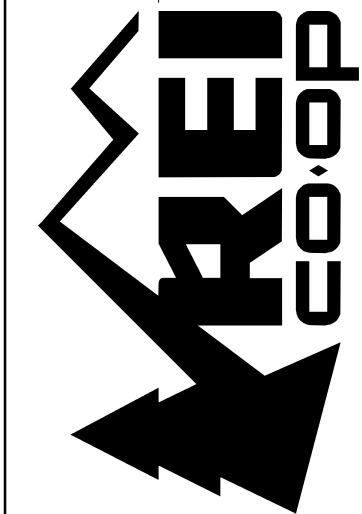
NTS.



CLIENT STORE NUMBER

235

CLIENT INFORMATION:



PROJECT INFORMATION:

CALLISONRTKL™

CallisonRTKL, Inc.
CallisonRTKL, Inc.
1402 Br. Ave. Suite
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006-125864.81

CONSULTANT INFORMATION:



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BELLEVUE, WA 98004
425 614-0949 / INFO@AU-ENG.COM
JOB NO: 21173

PROJECT INFORMATION:

REI-GLENWOOD SPRINGS

3216 S. GLEN AVENUE
GLENWOOD SPRINGS,
CO. 81601

SIGNATURES:

DRAWING REVISION LOG

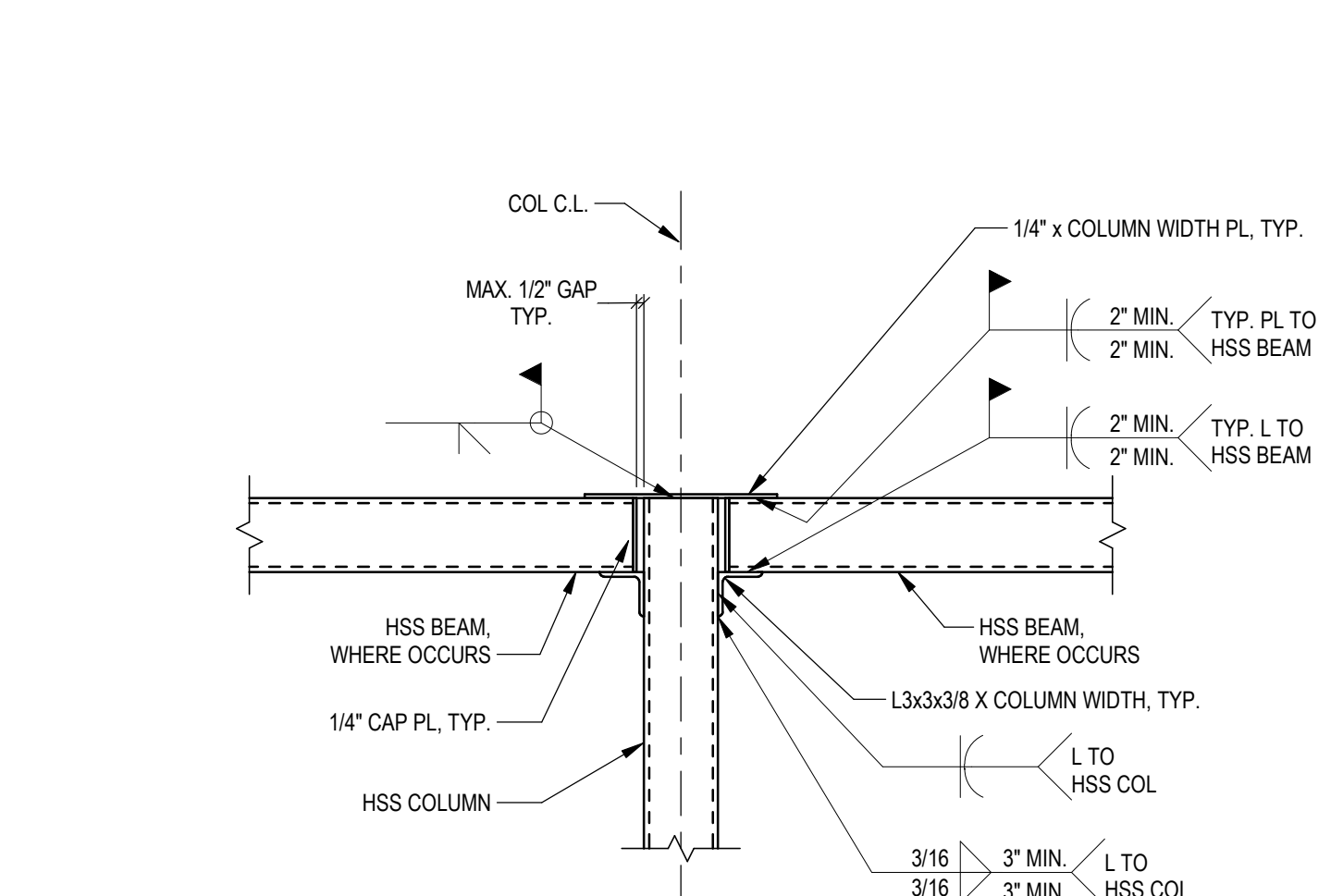
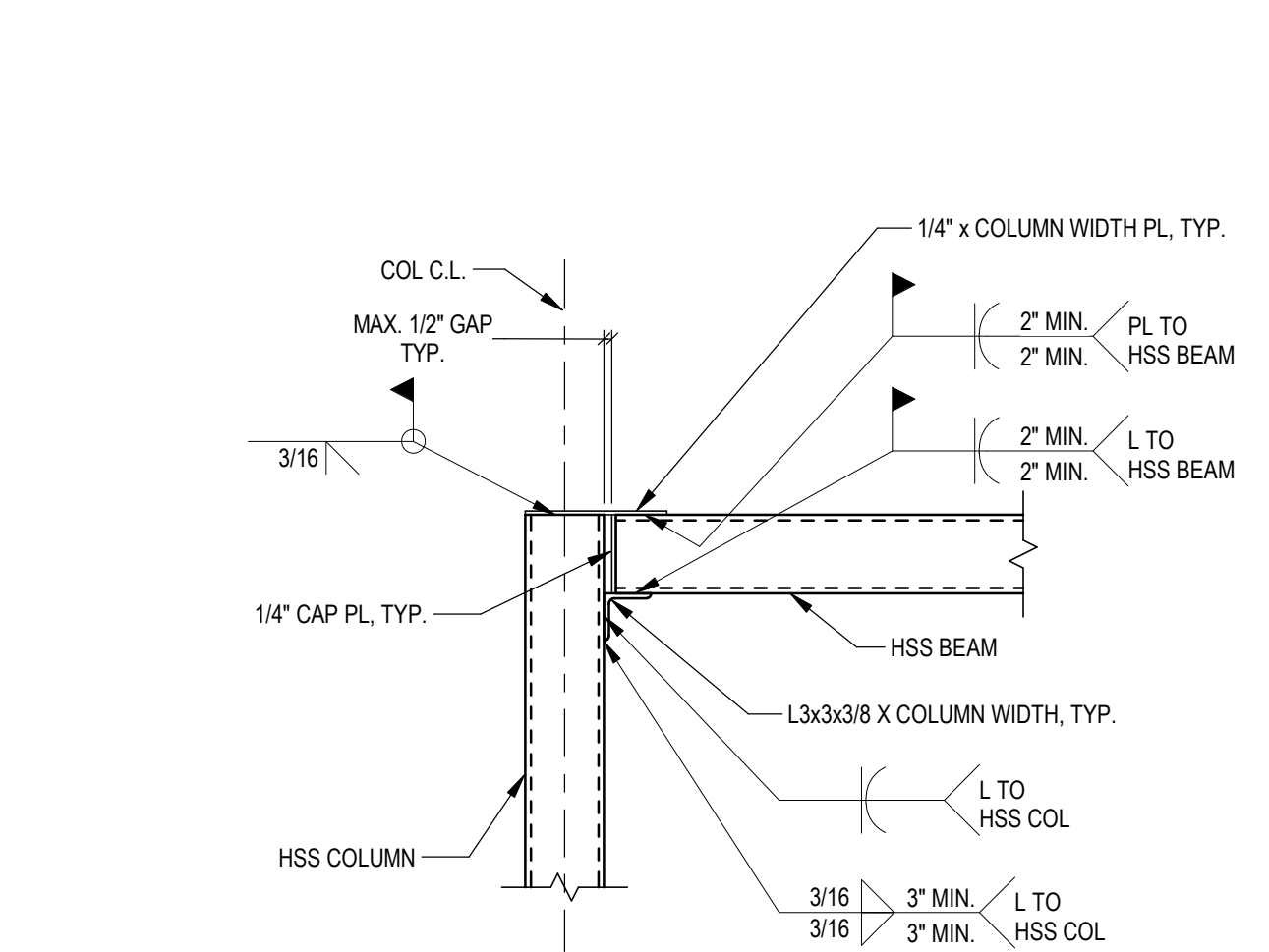
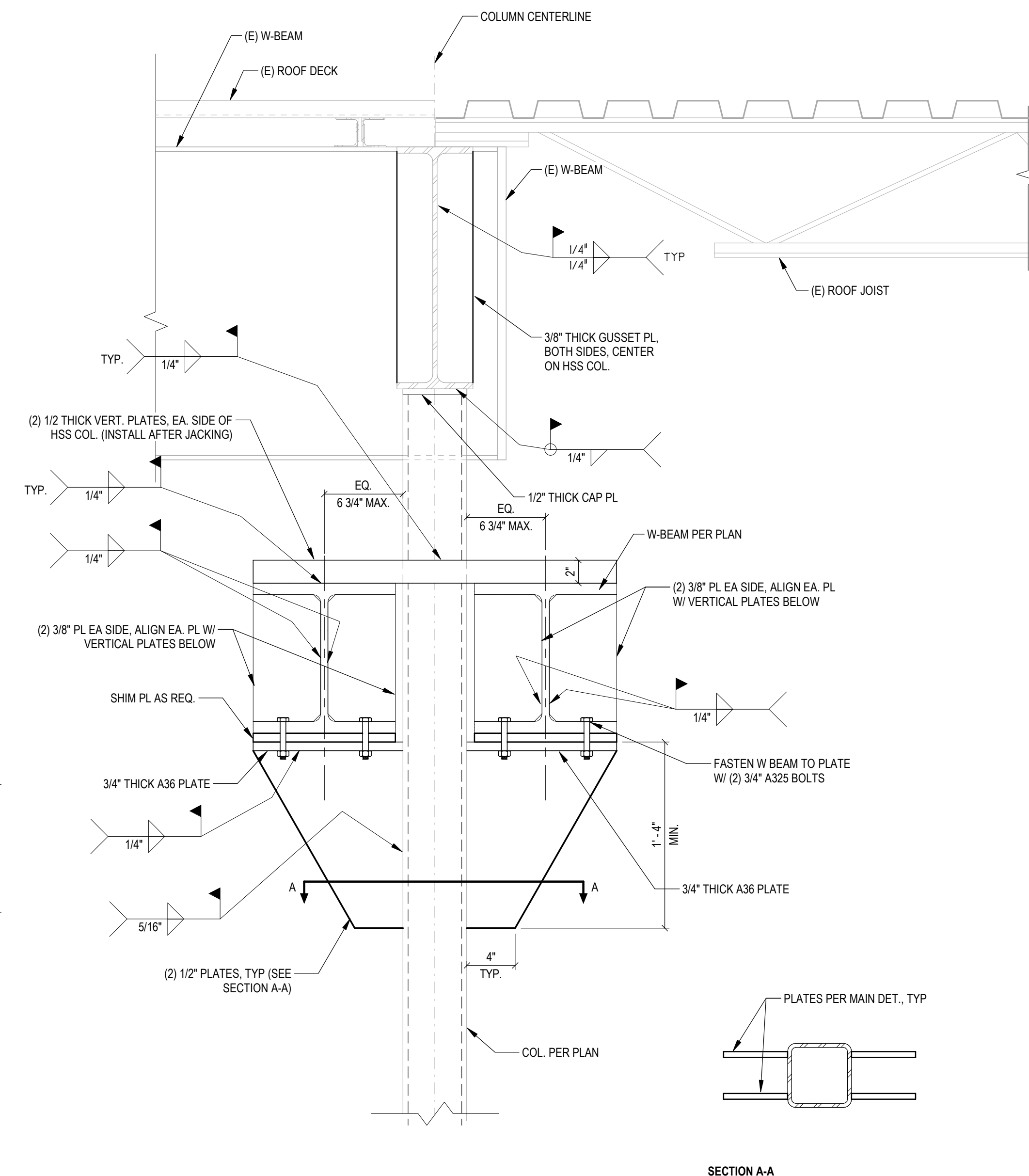
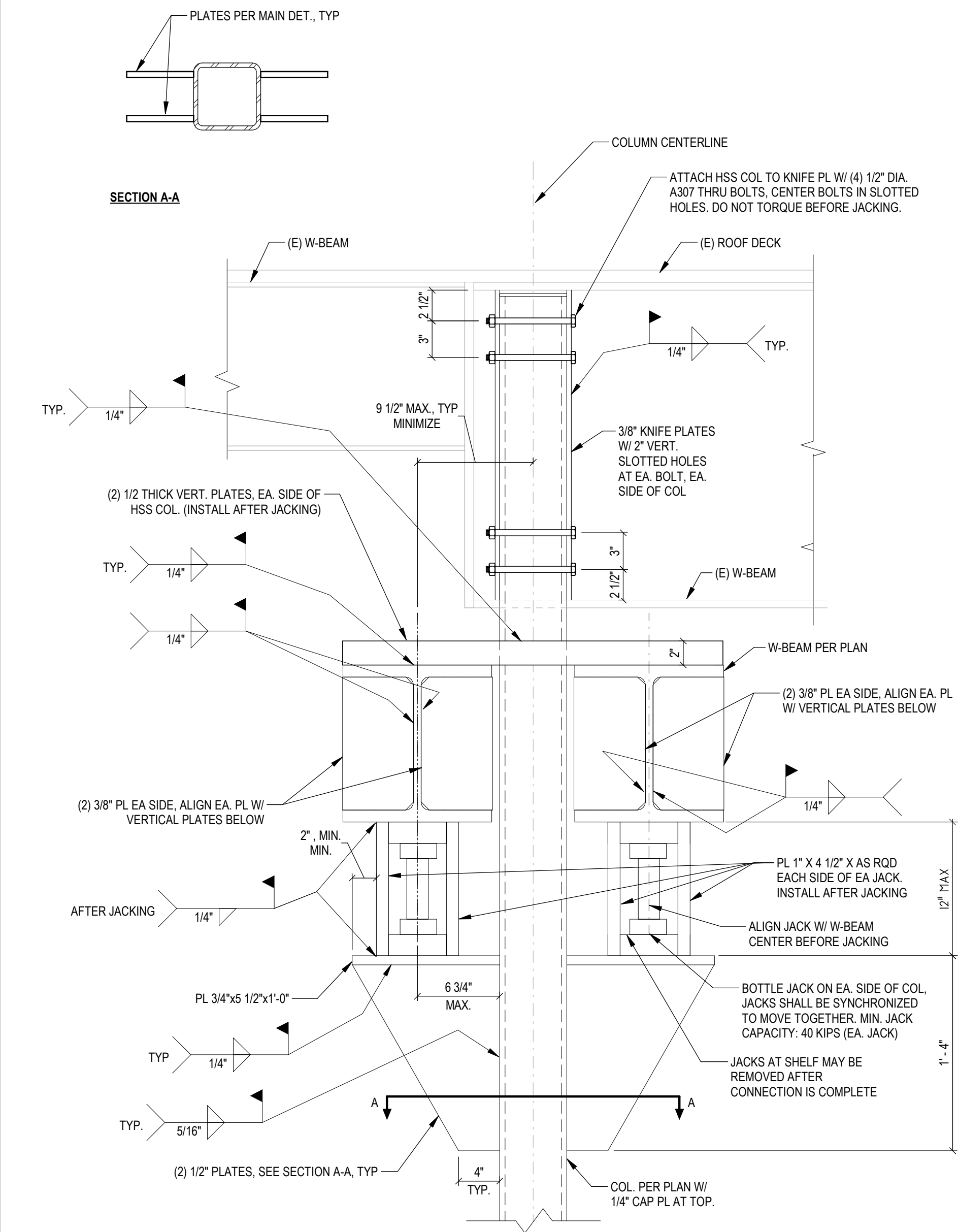
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11/08/21	BID SET	

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CONCRETE DETAILS

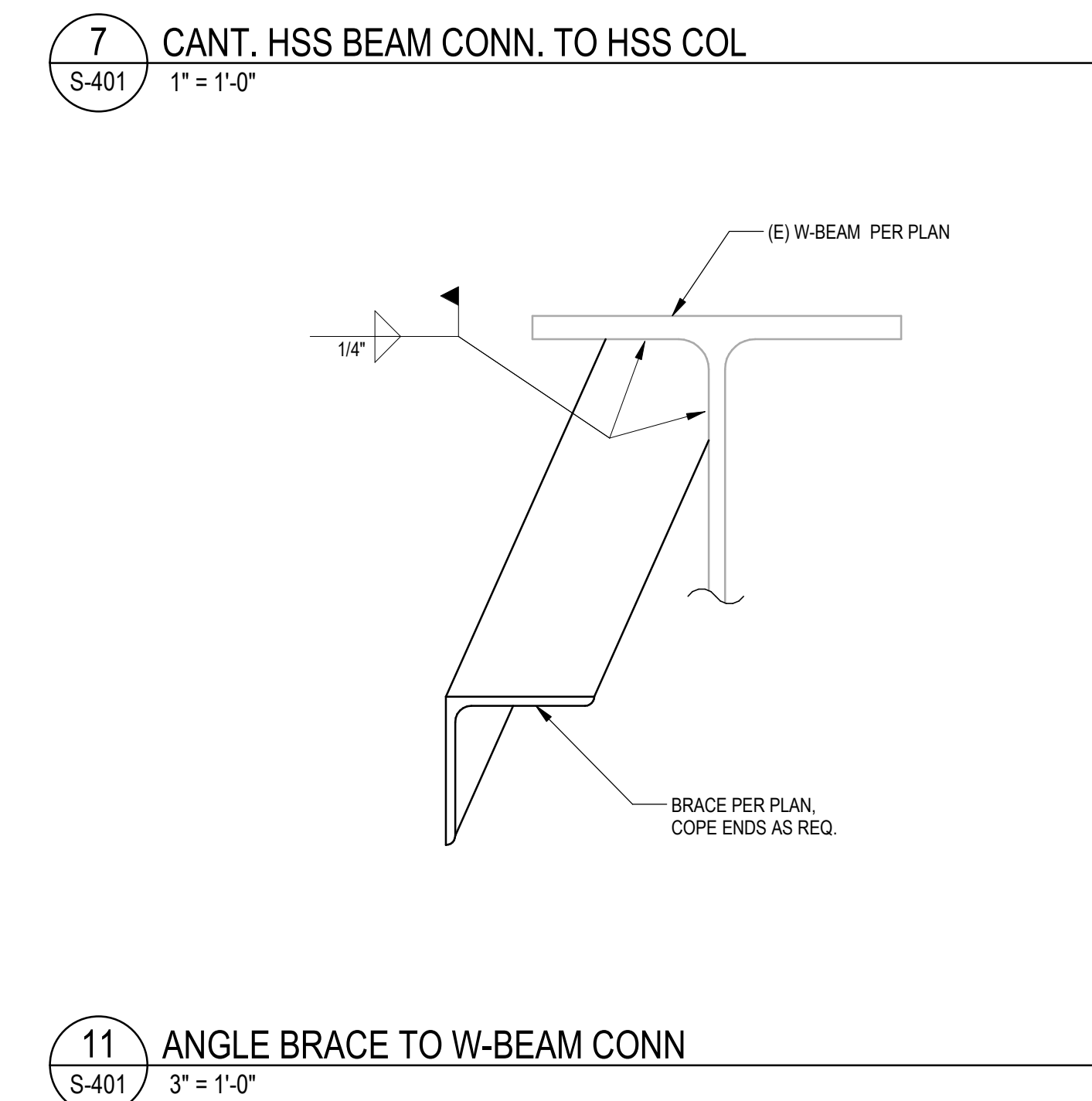
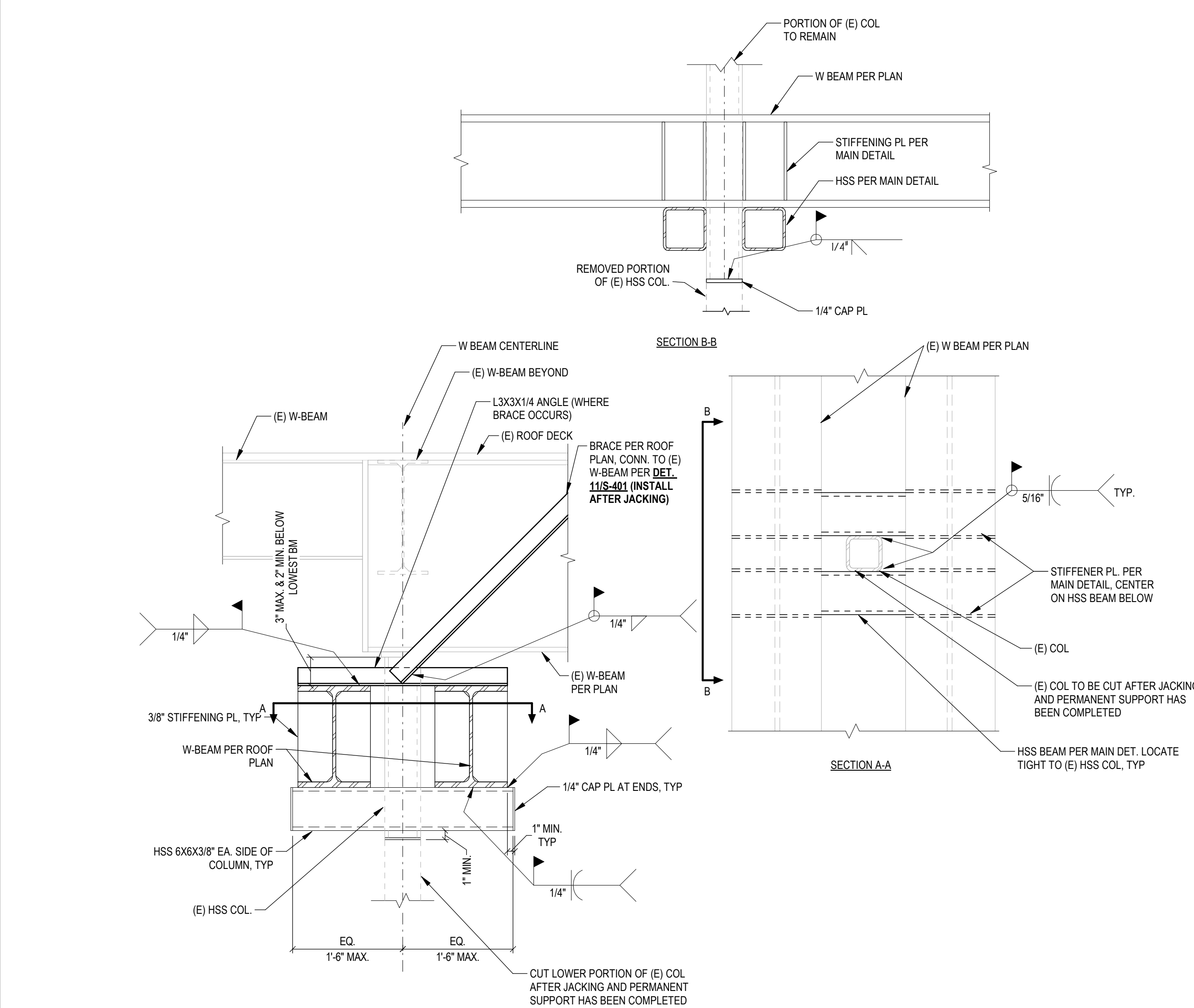
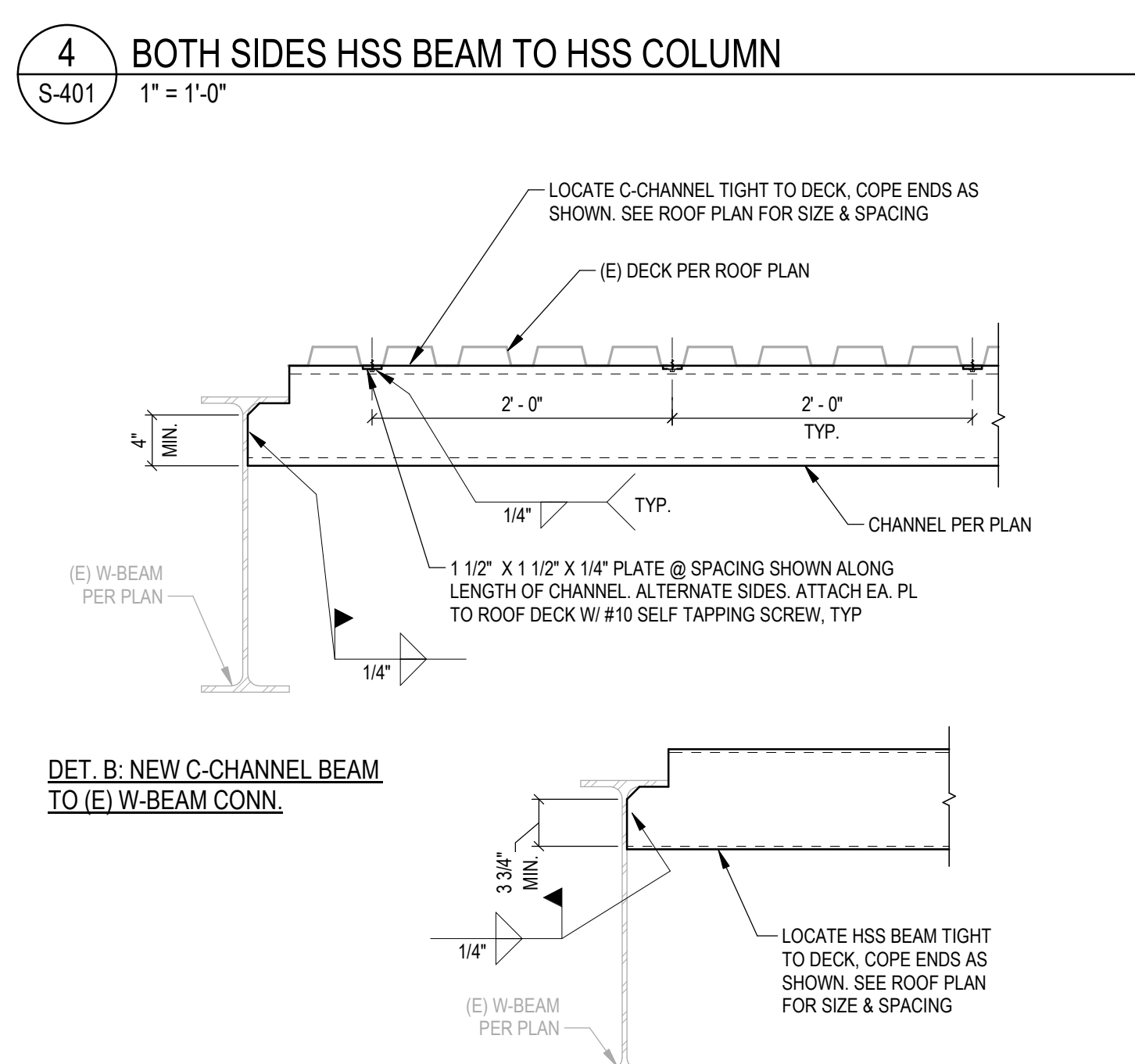
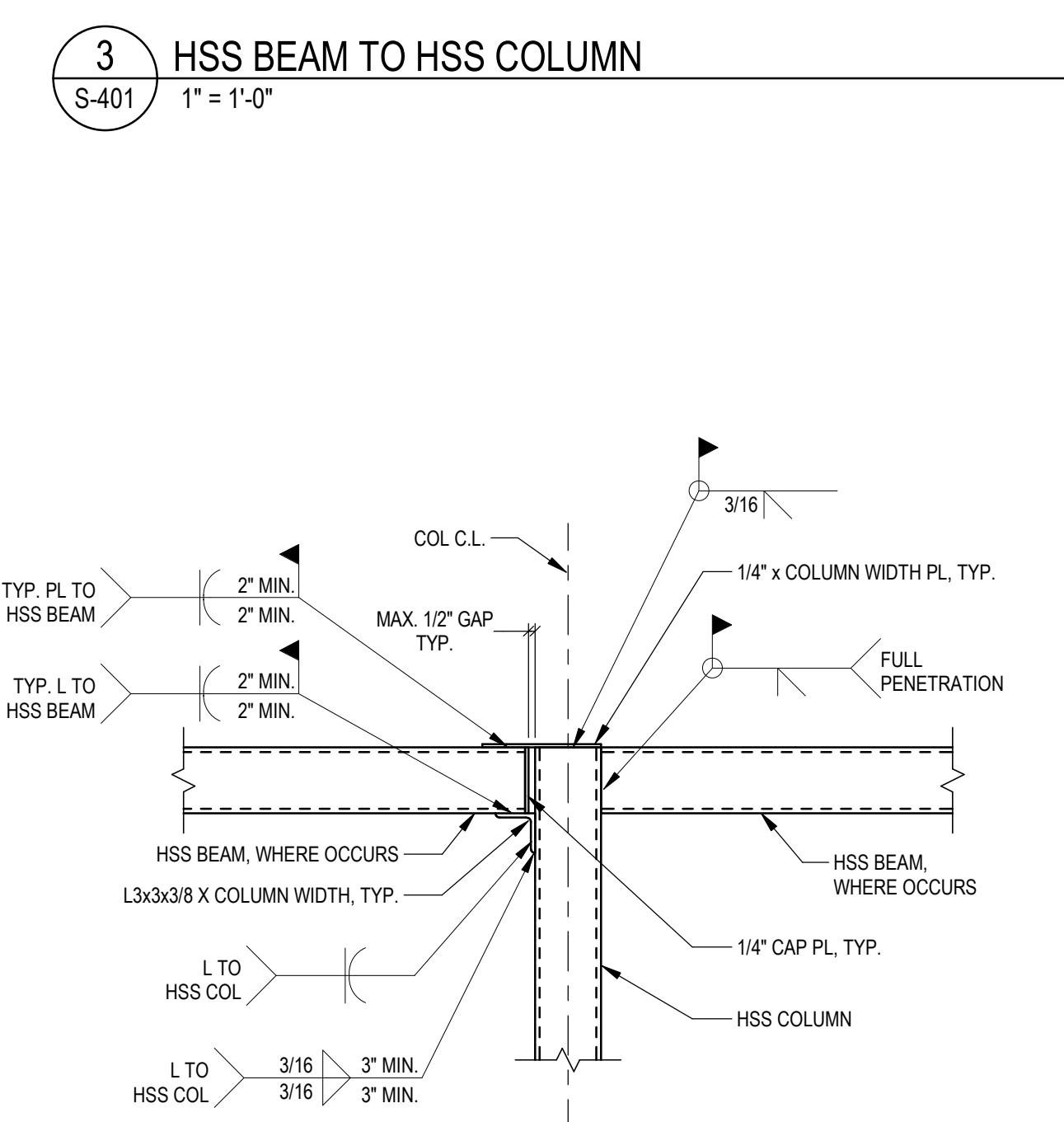
SHEET NUMBER:

S-301



5 TRANSFER GIRDER CONN. NEAR GRID J (DYNAMIC END)
S-401 1 1/2" = 1'-0"

6 TRANSFER GIRDER CONNECTION NEAR GRID G (STATIC END)
S-401 1 1/2" = 1'-0"



13 COLUMN CAPTURE CONNECTION AT GRID J
S-401 1" = 1'-0"

CLIENT STORE NUMBER

235

CLIENT INFORMATION

PROJECT INFORMATION

CALLISONRTKL

CallisonRTKL Inc.

1402 8th Ave Suite

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JOB NO: 21173

PROJECT INFORMATION

REI-GLENWOOD SPRINGS

3216 S. GLEN AVENUE

GLENWOOD SPRINGS, CO. 81601

SIGNATURES:

DATE

DESCRIPTION

11/05/21

PERMIT SET

11/08/21

BID SET

DRAWING SOURCE LOC

REV

DATE

DESCRIPTION

11/05/21

PERMIT SET

11/08/21

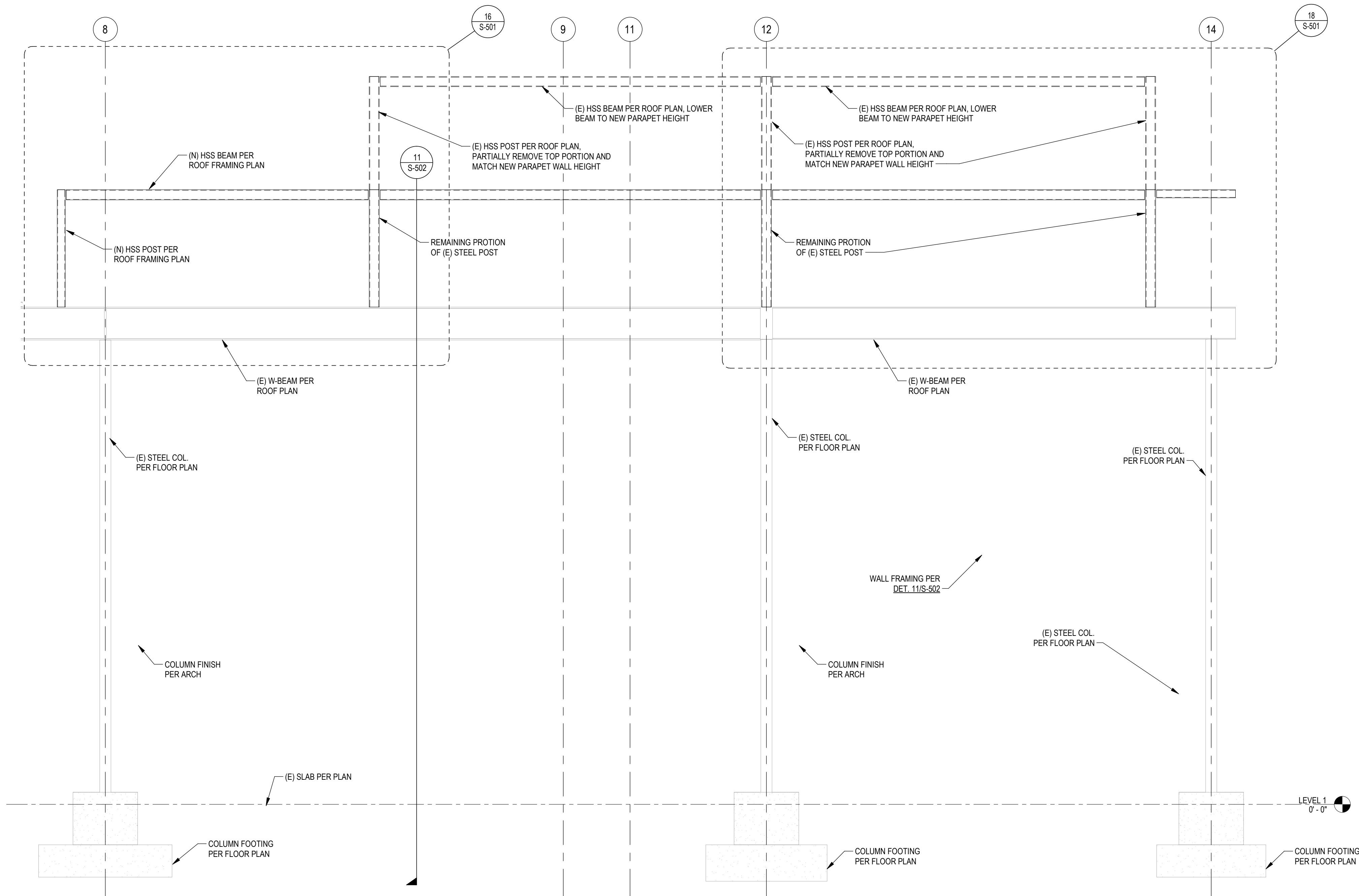
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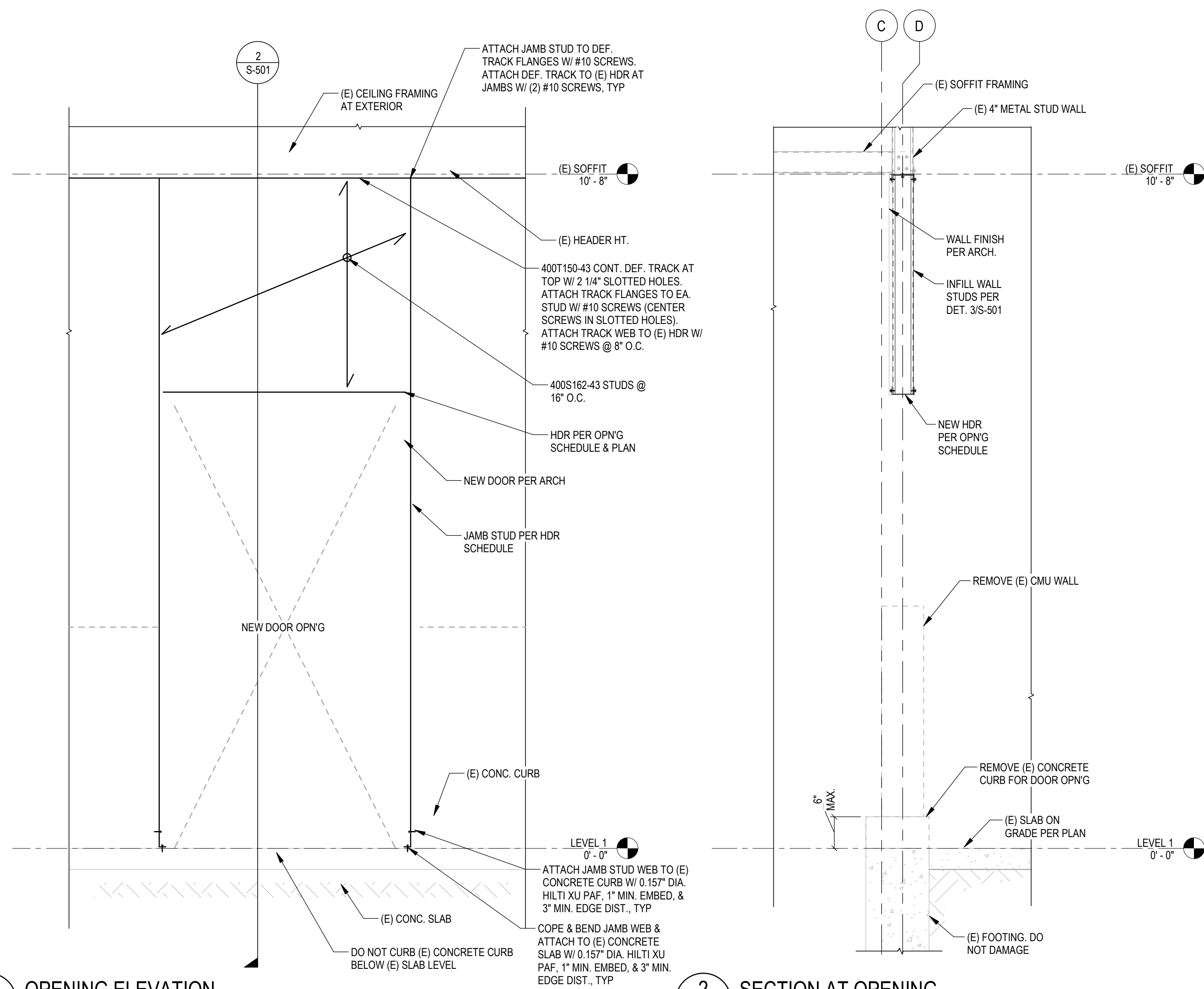
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S-401

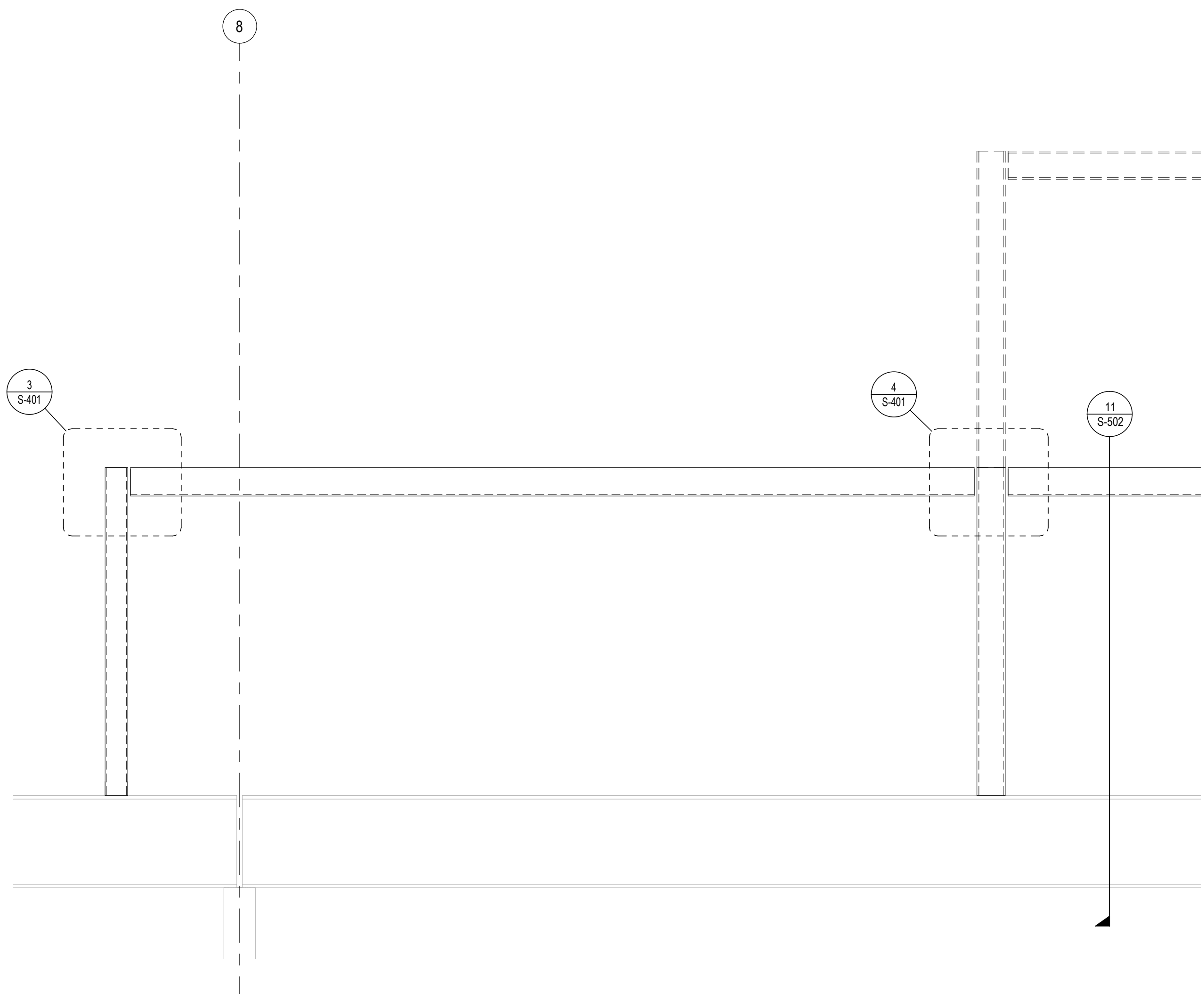


1 STOREFRONT ELEVATION
S-501 3/8" = 1'-0"

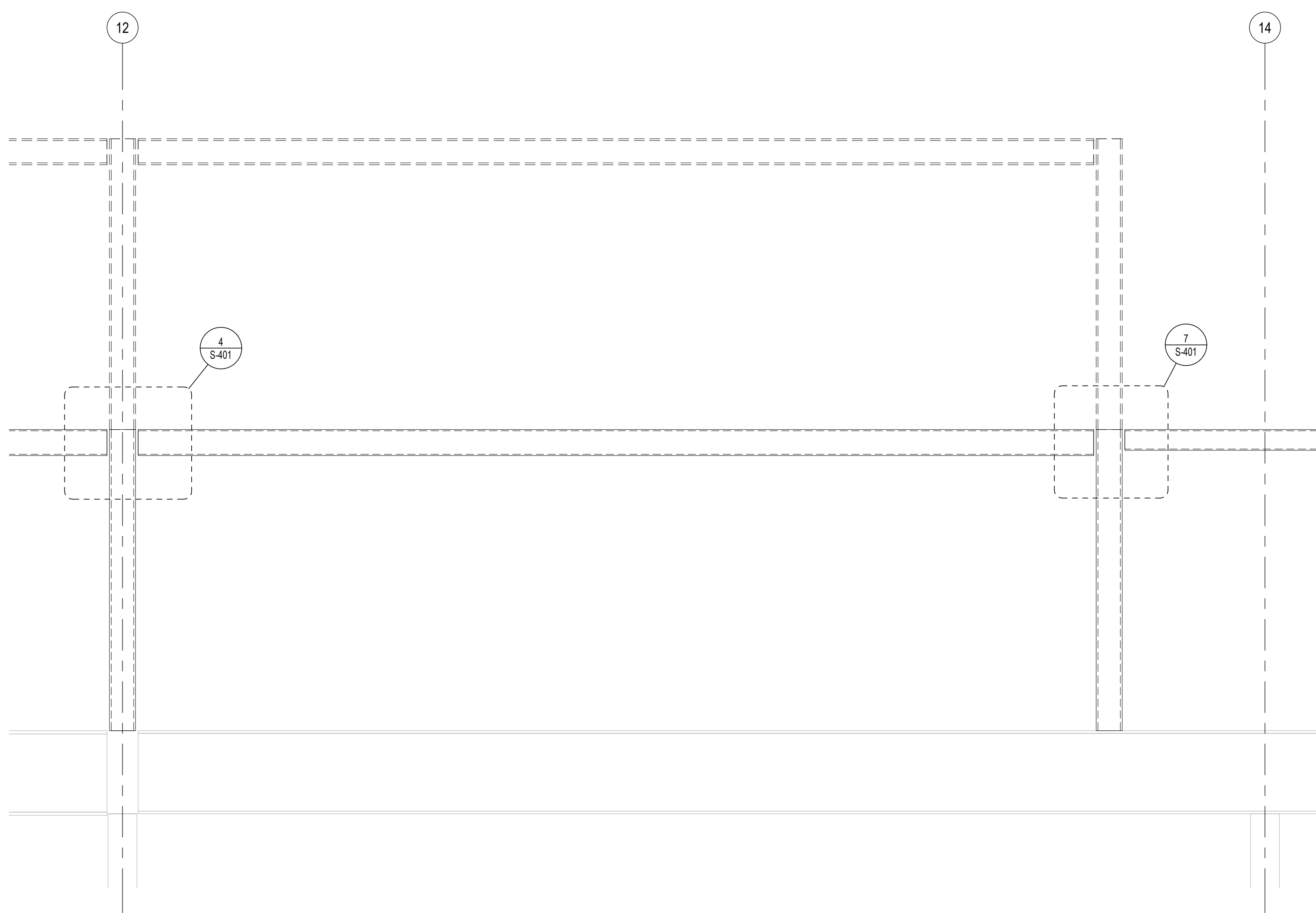


3 OPENING ELEVATION
S-501 3/4" = 1'-0"

2 SECTION AT OPENING
S-501 3/4" = 1'-0"



16 HSS FRAMING AT PARAPET LVL
S-501 3/4" = 1'-0"



18 HSS BEAMS AND POSTS AT PARAPET LVL
S-501 3/4" = 1'-0"

CLIENT STORE NUMBER

235

CLIENT INFORMATION

ARCHITECT INFORMATION

CONSULTANT INFORMATION

PROJECT INFORMATION

13228 NE 20th STREET, Ste. 100
BELLEVUE, WA 98004
425 614-0949 / INFO@AU-ENG.COM
JOB NO: 21173

SHEET TITLE

STOREFRONT ELEVATIONS

SHEET NUMBER

S-501

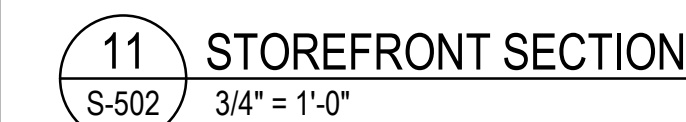
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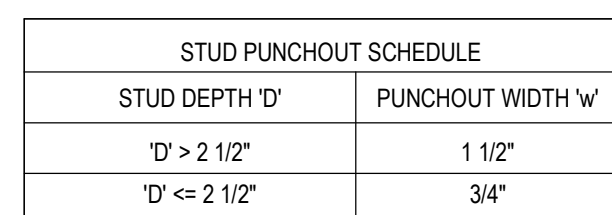
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11/05/21	11/05/21	PERMIT SET
11/08/21	11/08/21	BID SET

SIGNATURE/SEAL

3216 S. GLEN AVENUE
GLENWOOD SPRINGS,
CO. 81601

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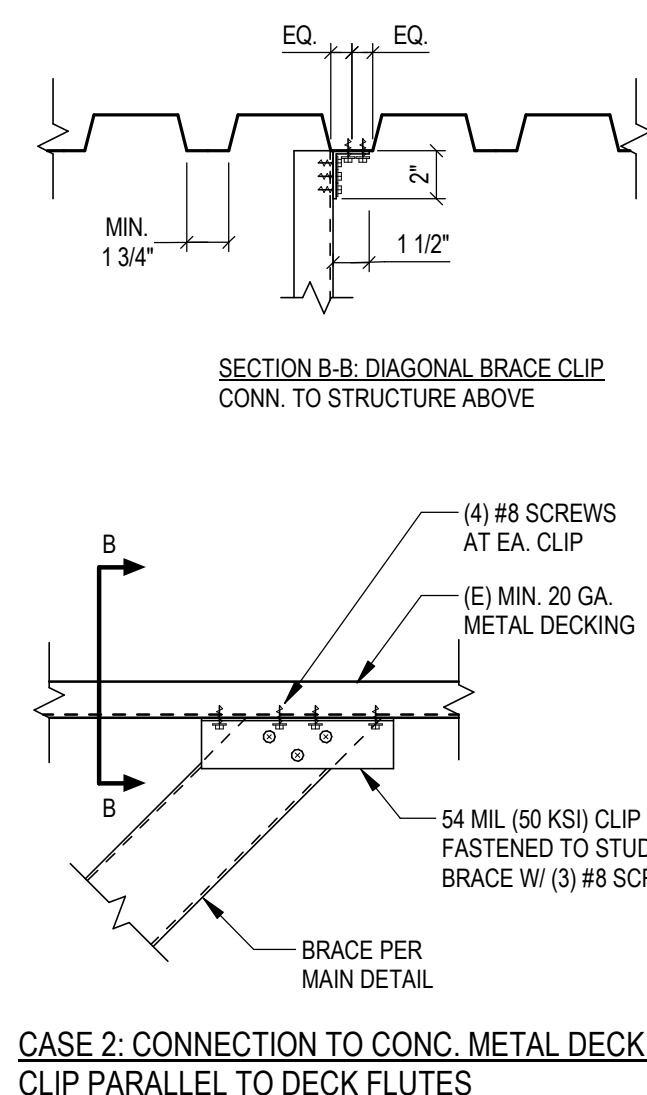




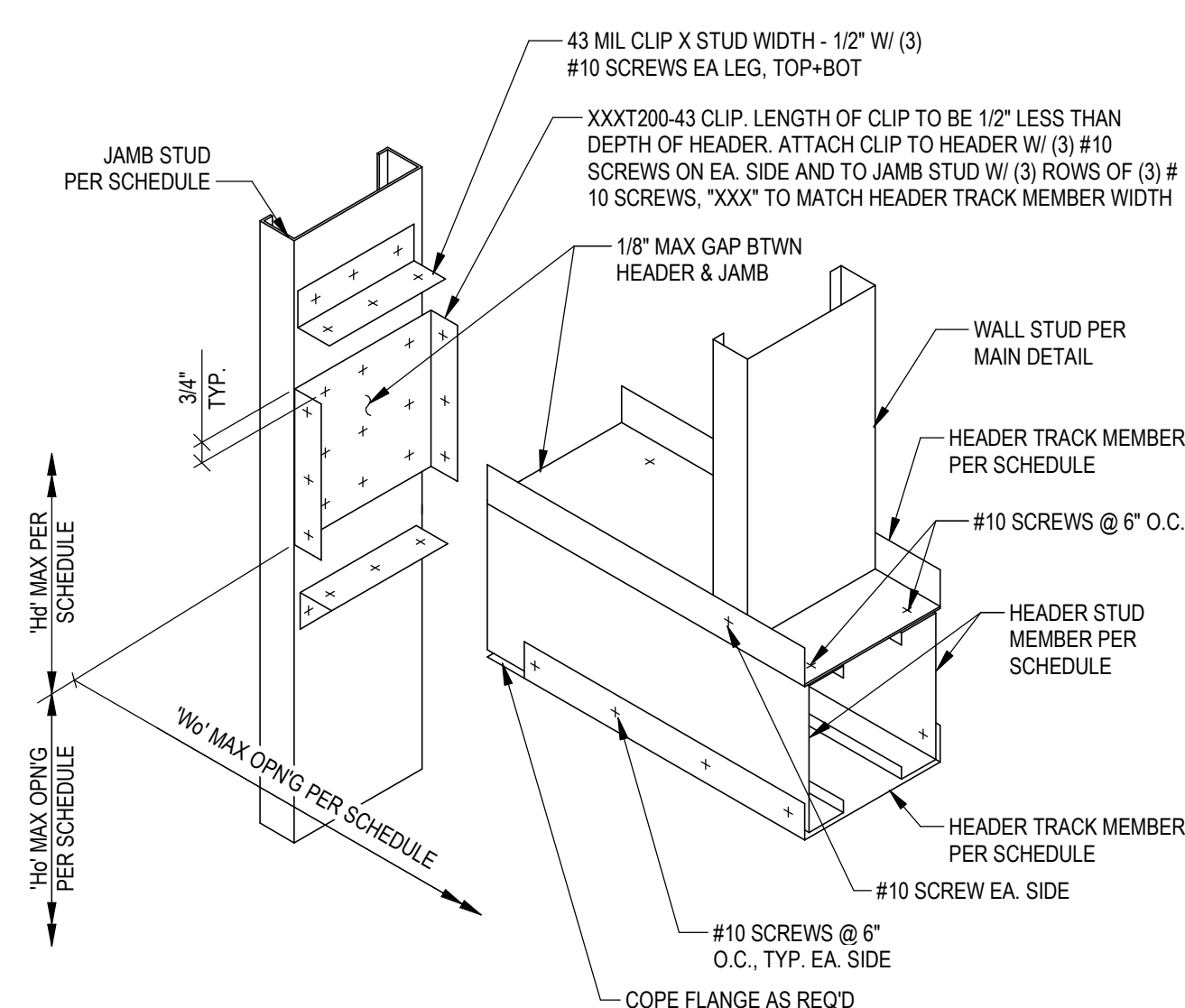
4 TYPICAL STUD PUNCHOUTS

7 DEFLECTION TRACK CONNECTION TO METAL DECK
S-601 1 1/2" = 1'-0"

12 WALL OPENING SCHEMATIC
S-601 1" = 1'-0"



16 BOX BEAM TO JAMB
S-601 NTS



17 JAMB BASE CONN. TO SLAB
S-601 1 1/2" = 1'-0"

13 WALL OPENING SCHEDULE

[illegible]

NOTES:
1. FOR BALANCE OF INFORMATION SEE DETAILS 12/S-601 & 14/S-601

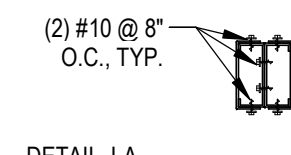
13 WALL OPENING SCHEDULE

14 WALL OPENING MEMBER SCHEDULE
S-601 1" = 1'-0"

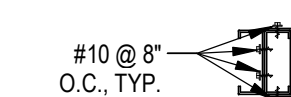
HEADER SCHEDULE			
HEADER	ASSEMBLY	MEMBERS	HEADER CONN. DET.
H1	PER CONN.	(2) 600S162-33 (33 KSI) + (2) 600T150-33 (33 KSI)	16/S-601
H2	-	400T150-54 (50 KSI)	11/S-601
-	-	-	-
-	-	-	-

SILL SCHEDULE			
SILL	ASSEMBLY	MEMBERS	SILL CONN. DET
-	-	-	-
-	-	-	-

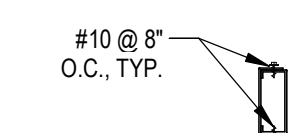
JAMB SCHEDULE				
JAMB	ASSEMBLY	MEMBERS	BOTTOM CONN.	TOP CONN.
J1	-	600S162-43	17/S-601	18/S-601
J2	J-C	600S162-43 (33KSI) + 600T150-43 (33KSI)	17/S-601	18/S-601
J3	J-C	400S162-43 (33KSI) + 400T150-43 (33KSI)	3/S-501	3/S-501
-	-	-	-	-



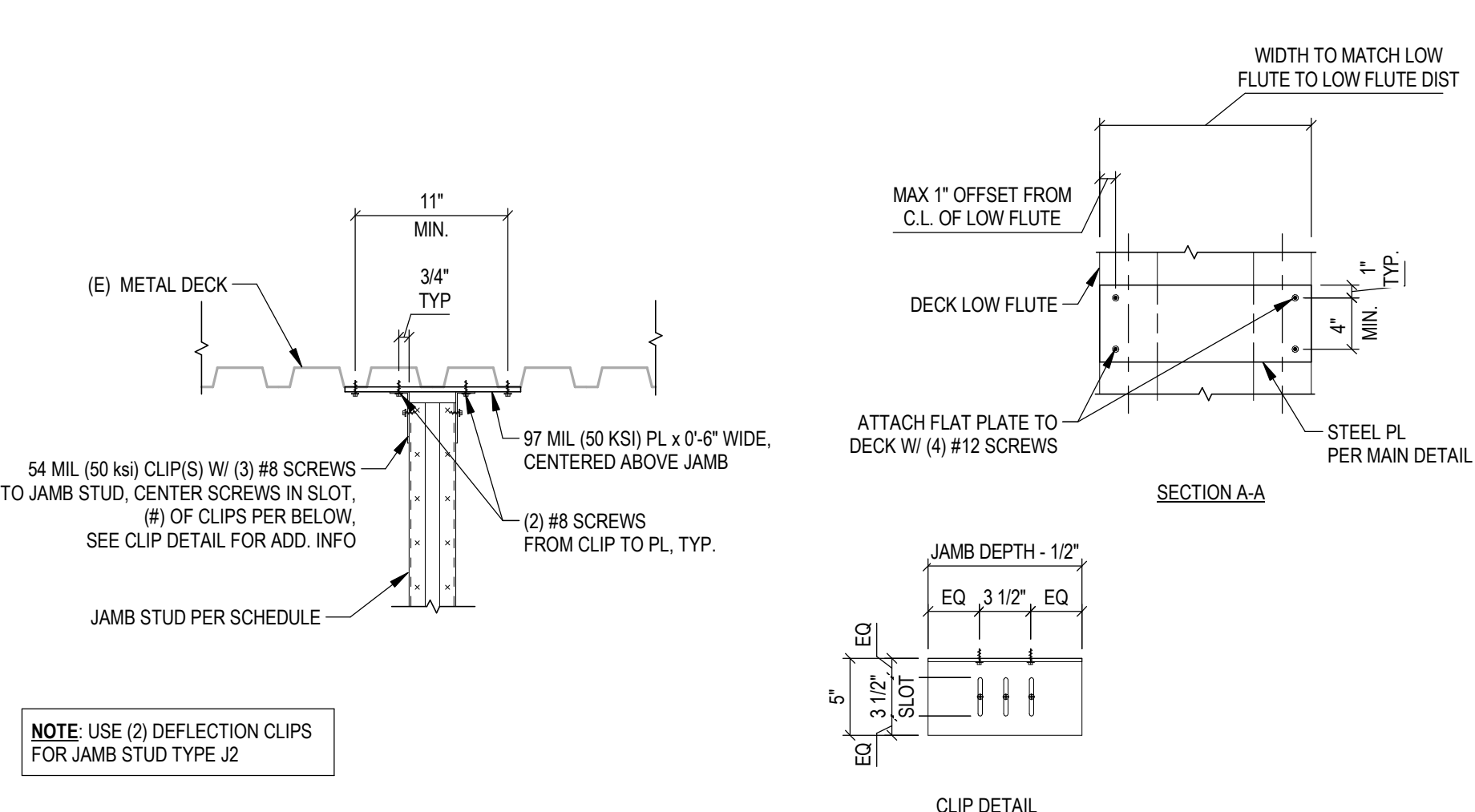
DETAIL J-4
JAMB STUD SECTION
MEMBER SIZES PER SCHEDULE



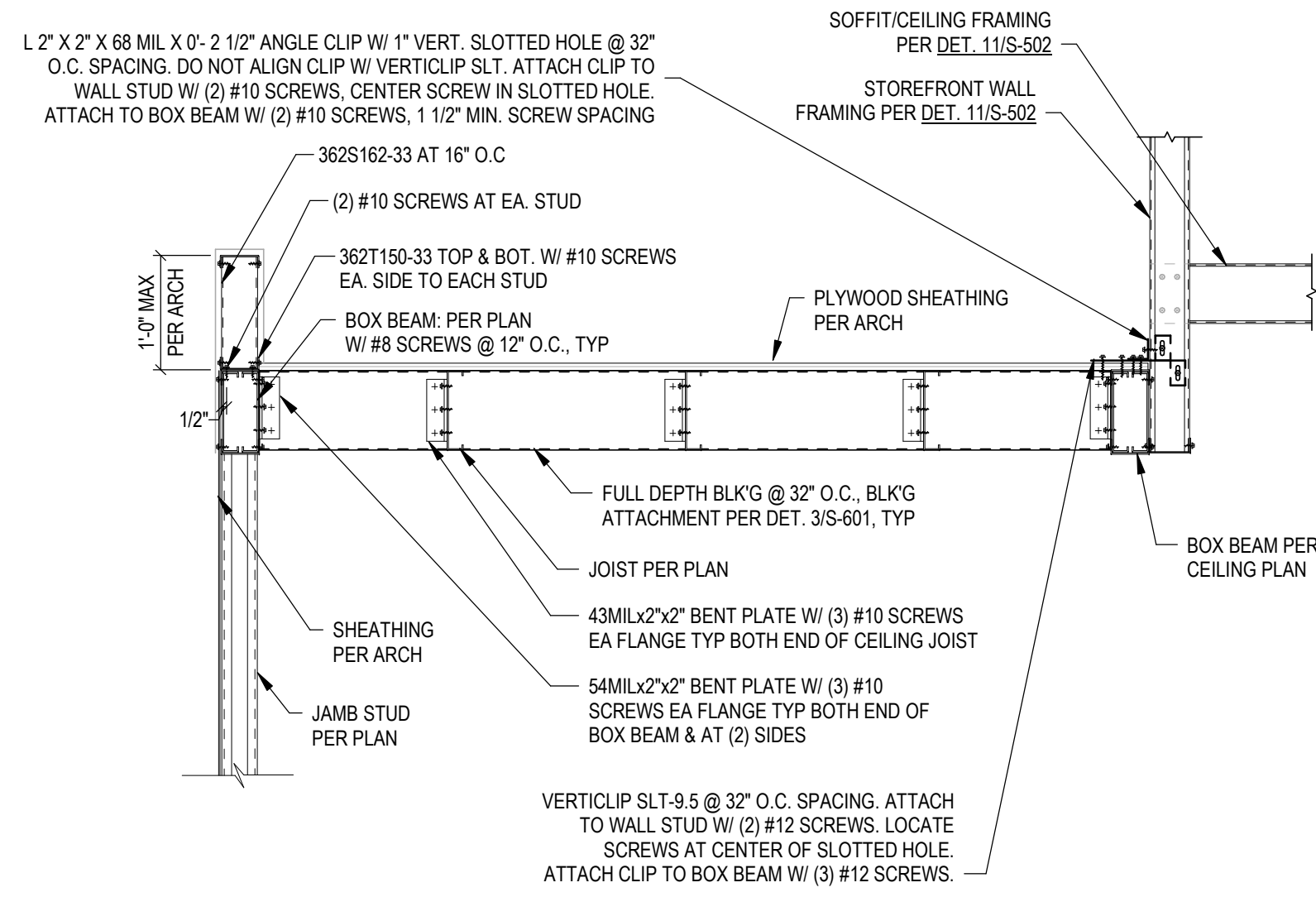
DETAIL J-B
JAMB STUD SECTION
MEMBER SIZES PER SCHEDULE



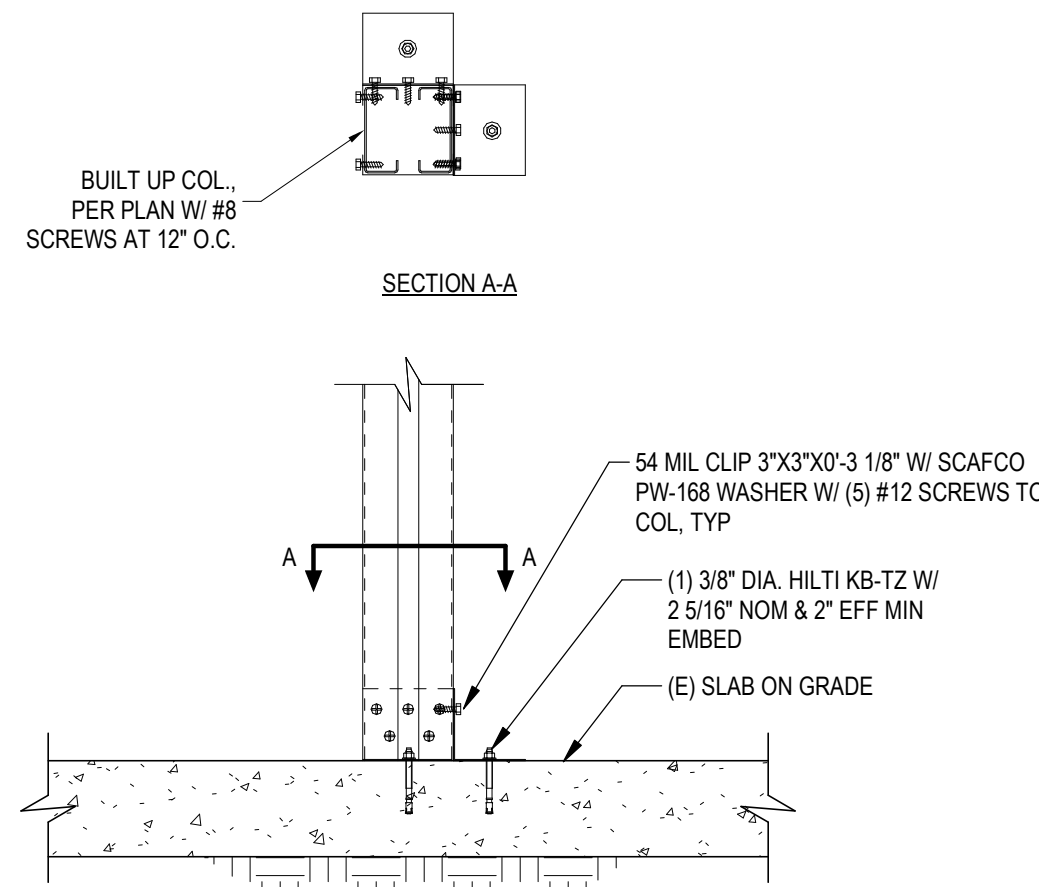
DETAIL J-C
JAMB STUD SECTION
MEMBER SIZES PER SCHEDULE



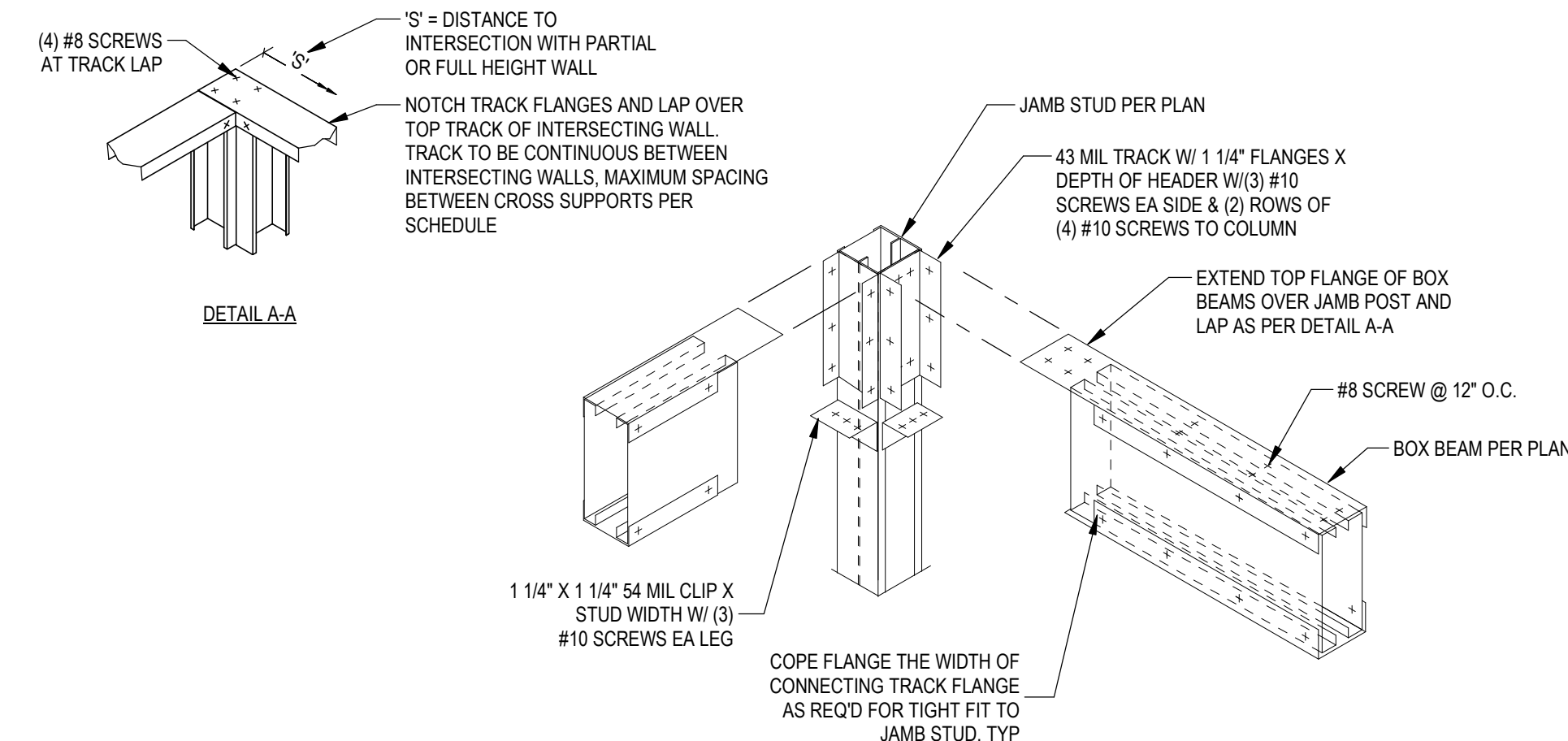
18 JAMB STUD TOP CONN. TO STRUC
S-601 1" = 1'-0"



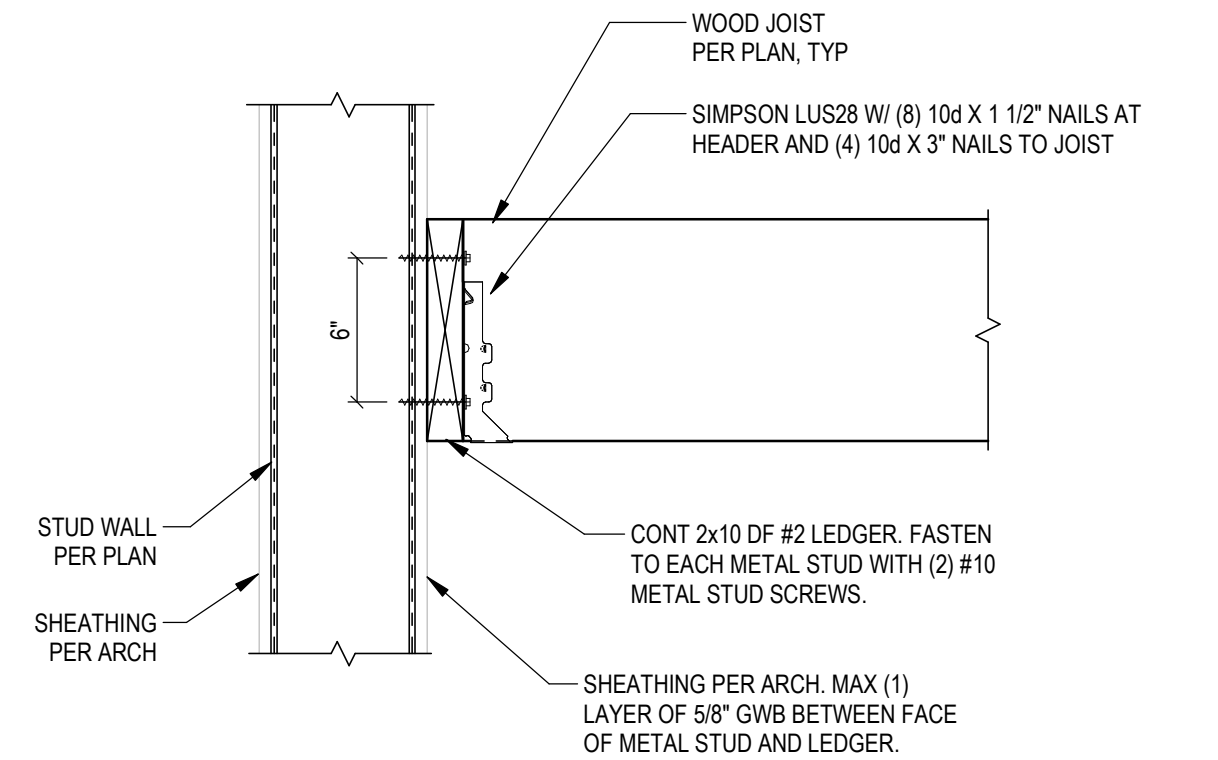
1 VESTIBULE SECTION
S-602 3/4" = 1'-0"



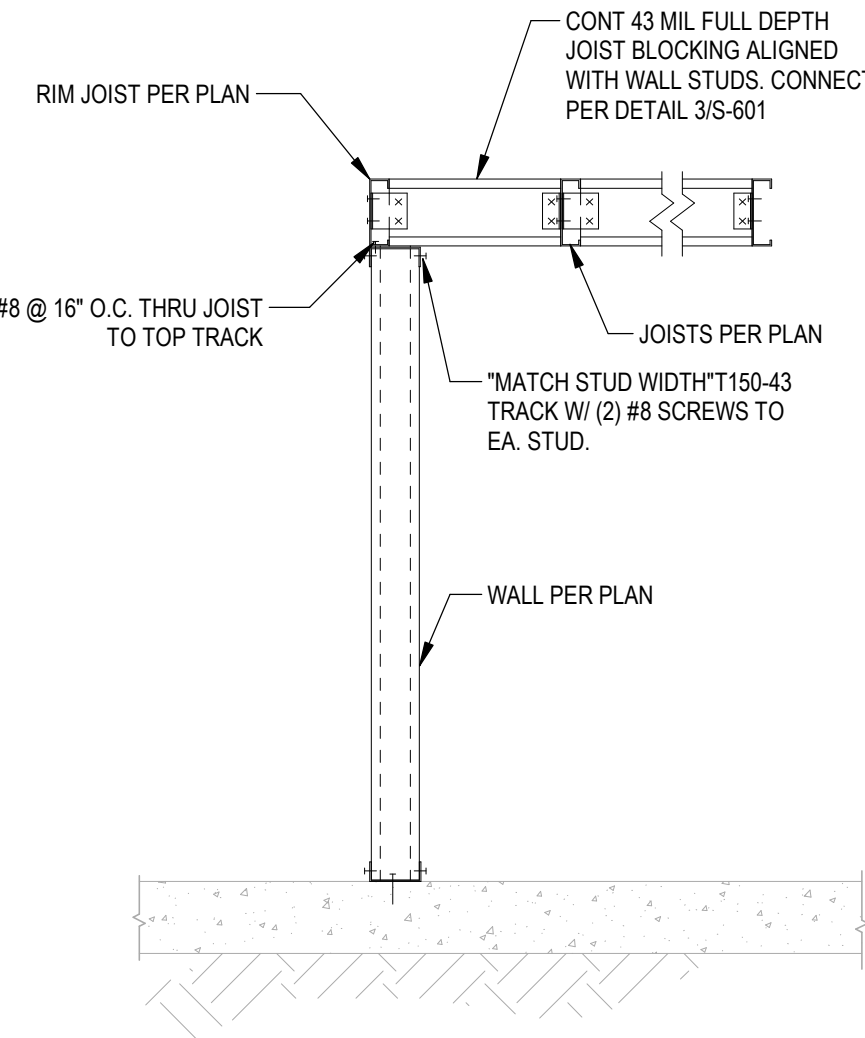
3 BUILT UP COL. TO (E) SLAB
S-602 1 1/2" = 1'-0"



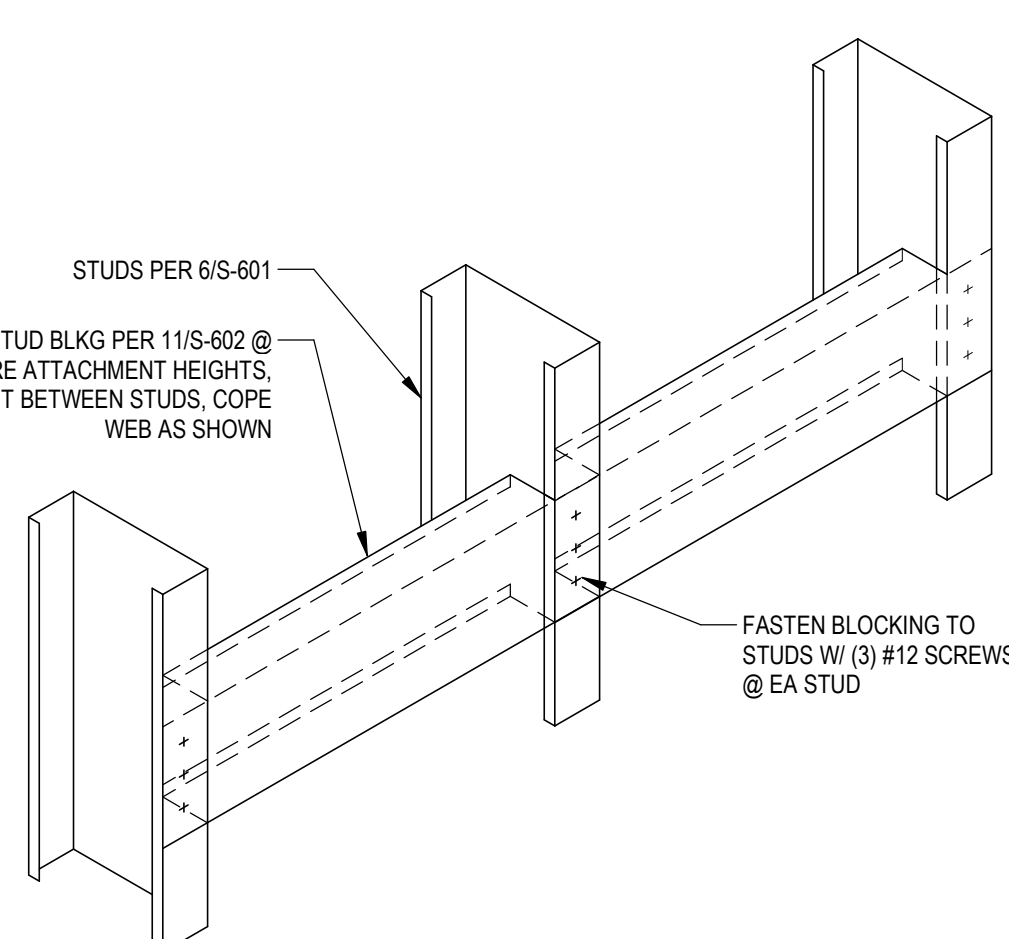
4 JAMB STUD ATTACHMENT TO BOX BEAM
S-602 1" = 1'-0"



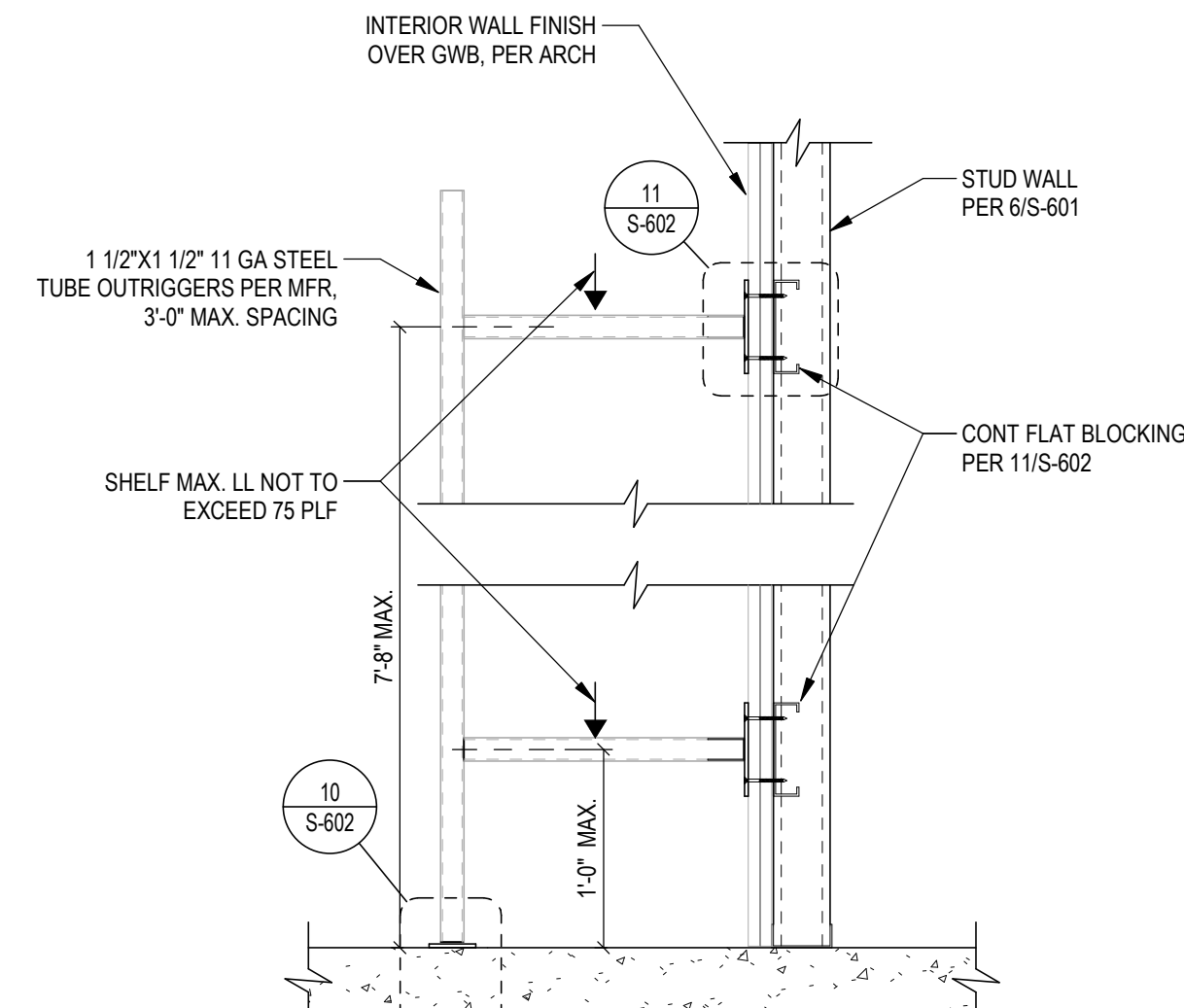
6 WOOD JOIST CONNECTION
S-602 1 1/2" = 1'-0"



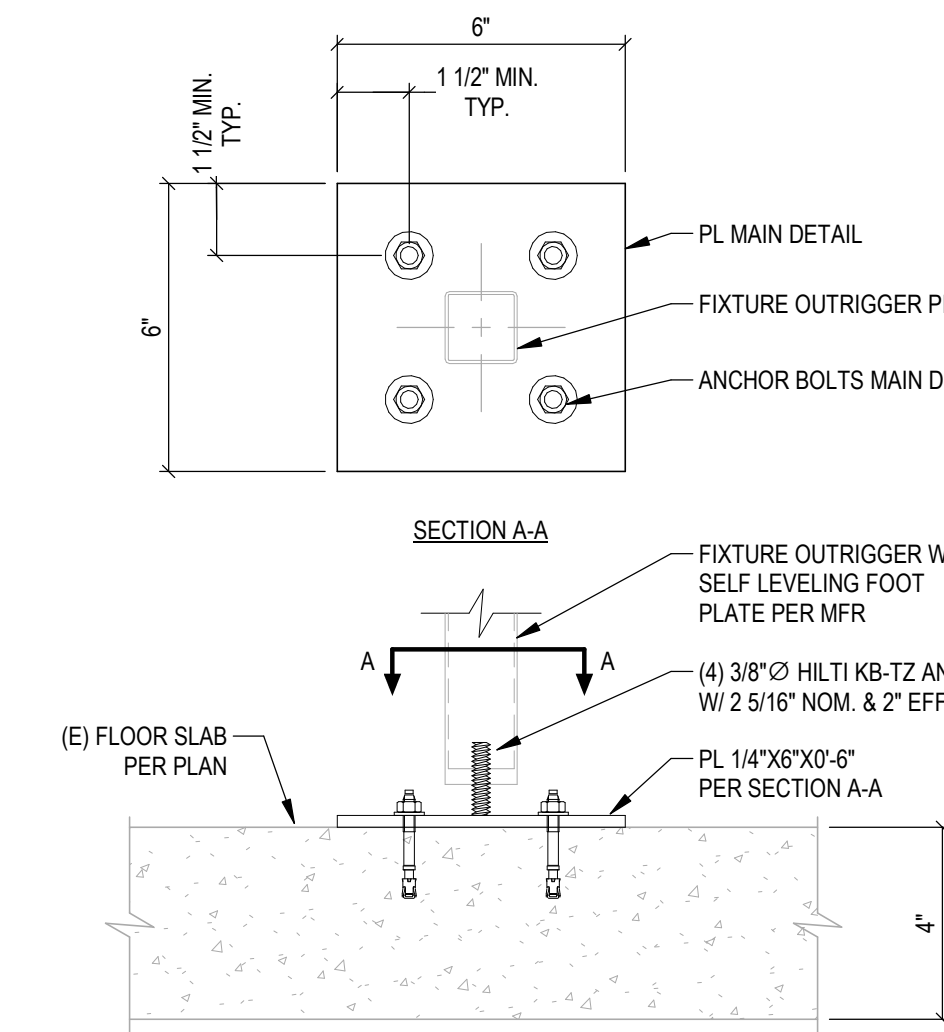
7 STEEL STUD JOIST CONFIGURATION
S-602 3/4" = 1'-0"



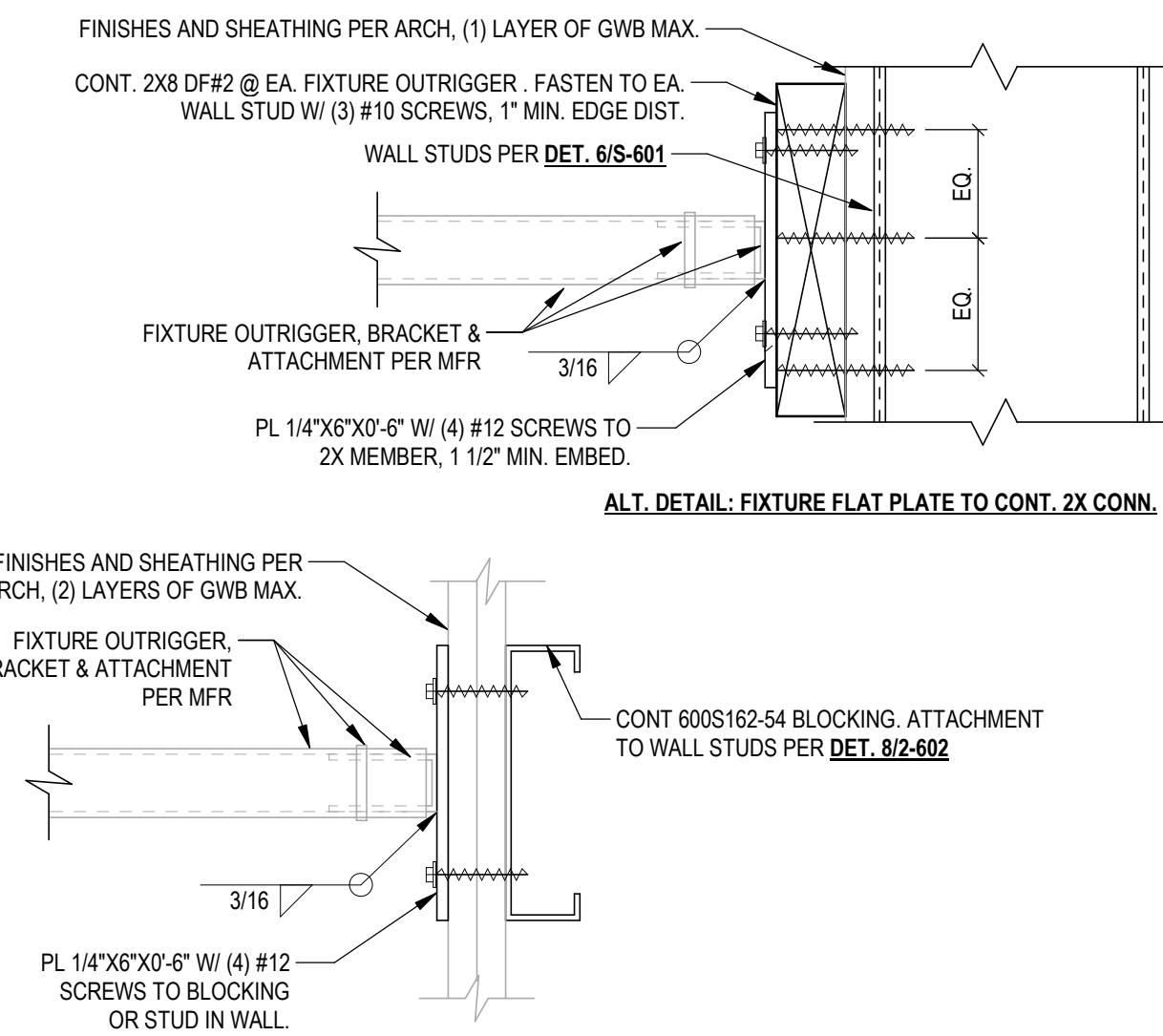
8 FIXTURE SUPPORT BLOCKING ATTACHMENT
S-602 3/4" = 1'-0"



9 TYPICAL PERIMETER FIXTURE CONN.
S-602 1" = 1'-0"

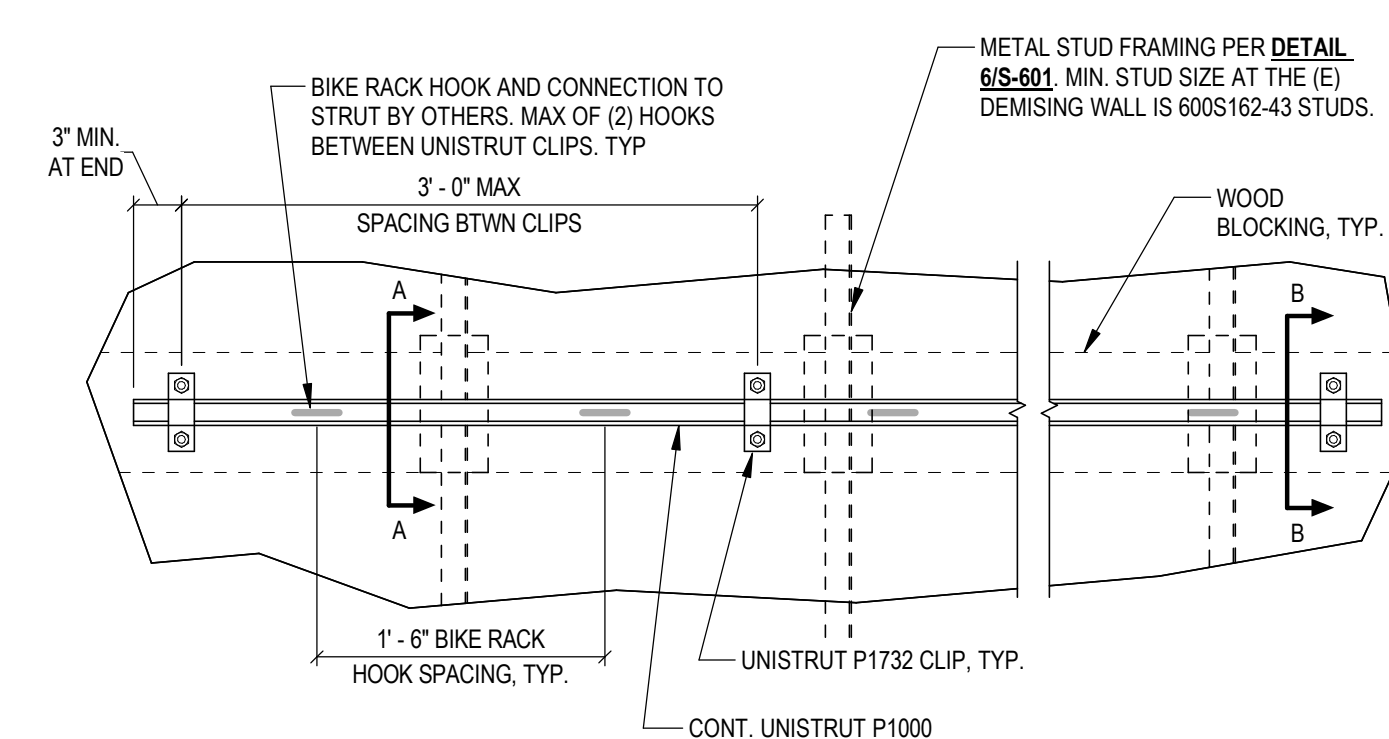


10 PERIMETER FIXTURE ATTACHMENT TO SLAB
S-602 3\"/>



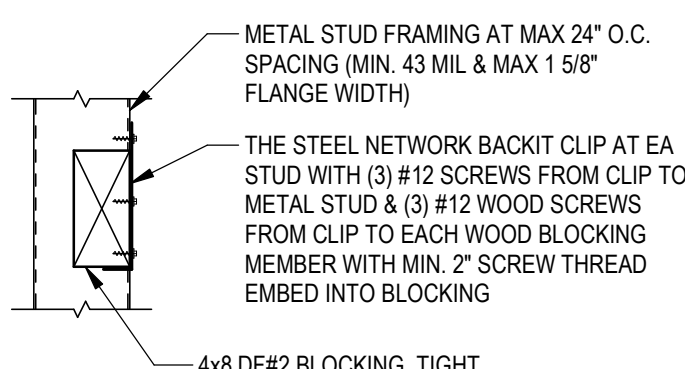
DETAIL A: FIXTURE FLAT PLATE TO BLK'S INSIDE WALL CONN.

11 PERIMETER FIXTURE ATTACHMENT TO WALL STUDS
S-602 3\"/>

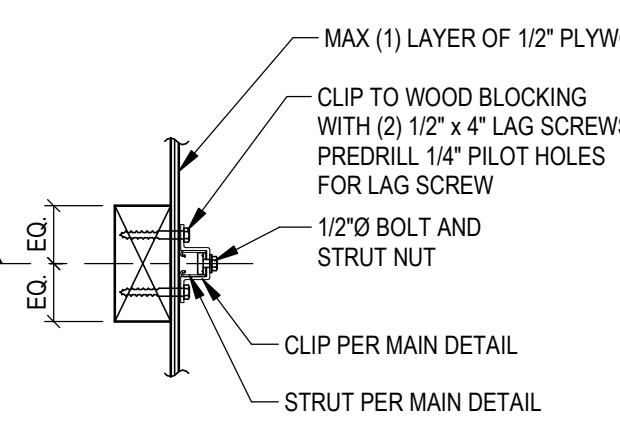


- NOTES:
1. MAX (1) BIKE PER HANGER
 2. MAX WEIGHT OF BIKE: 35 LBS
 3. MAX. ECE ECCENTRICITY FROM WALL FACE 6 5/8"

12 WALL MOUNTED BIKE RACK
S-602 1\"/>



SECTION A-A: BLKG TO MTL STUD CONN

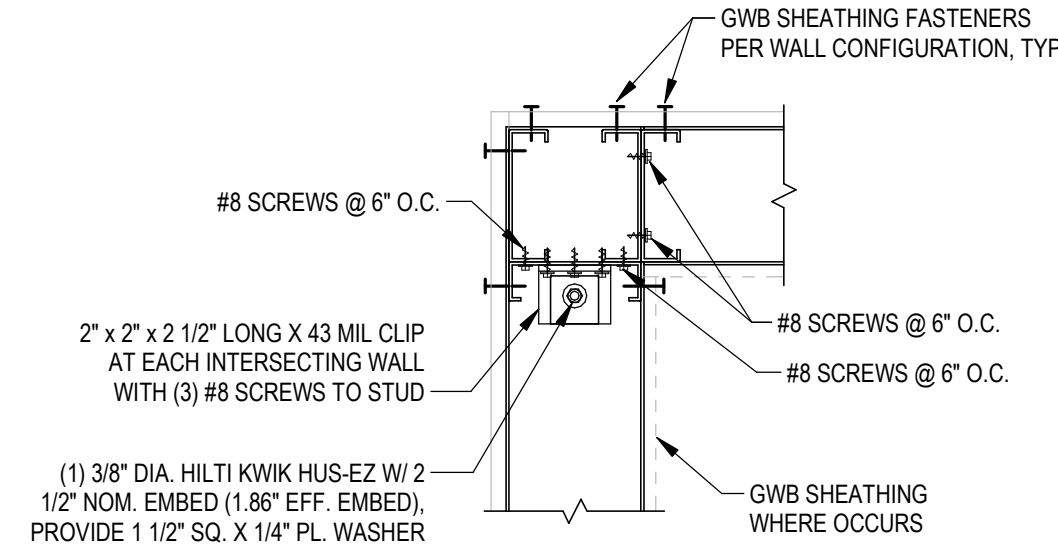


SECTION B-B: CLIP TO WOOD BLKG CONN

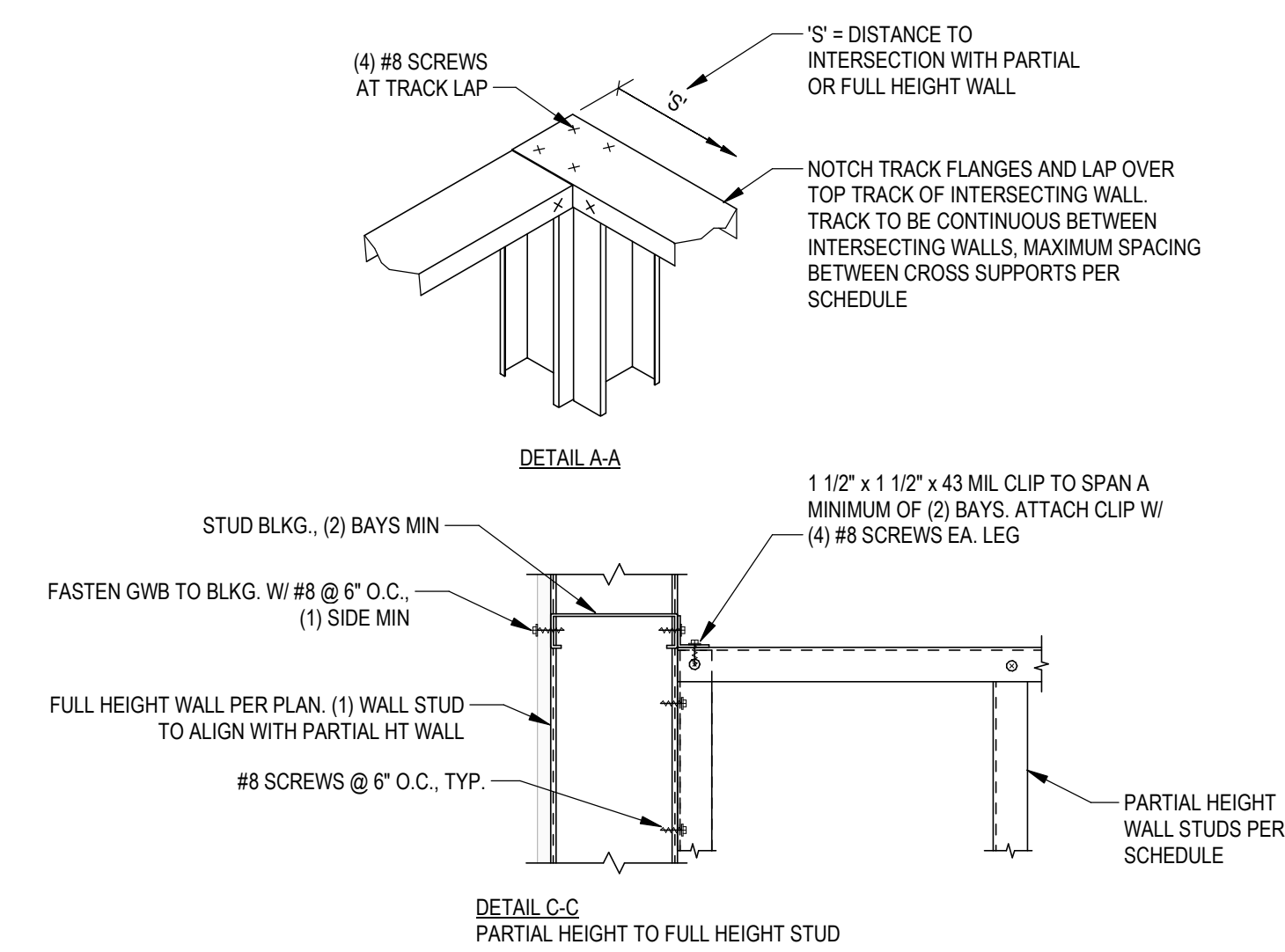
STANDALONE PARTIAL HEIGHT PARTITION WALL SCHEDULE					
WALL TYPE (LAYOUT PER PLAN)	STUD SIZE	MAX STUD O.C. SPACING	MAX WALL HEIGHT 'H'	TOP TRACK	MAX TOP TRACK SPAN 'S'
PH - 6	600S162-33	16"	16'-0"	600T150-54	16'-0"
PH - 3	362S162-33	16"	16'-0"	362T150-54	12'-0"

STANDALONE PARTIAL HEIGHT WALL NOTES:

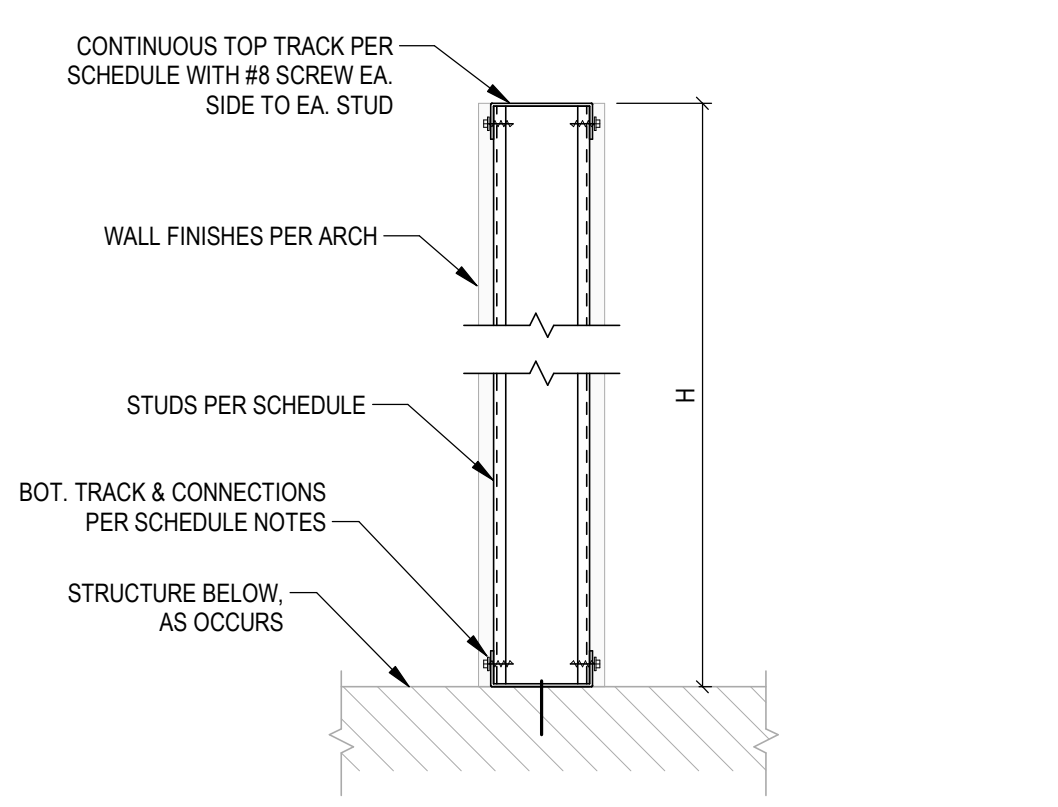
- ALL WALLS TO BE BLOCKED / BRACED PER 1/S-601 OR 2/S-601.
- SEE DETAIL 4/S-601 FOR TYPICAL STUD PUNCHOUTS.
- SEE DETAIL A-A FOR DEFINITION OF TOP TRACK SPAN 'S'.
- SEE DETAIL A-A' & DETAIL B-B' FOR FRAMING AT CORNER CONDITIONS.
- SEE DETAIL C-C FOR TOP TRACK ATTACHMENT TO FULL HEIGHT WALL.
- SEE 12/S-601 FOR DETAILING AT DRINKINS.
- SEE 6/S-601 FOR BOTTOM TRACK & ATTACHMENT TO SLAB ON GRADE.



DETAIL B-B

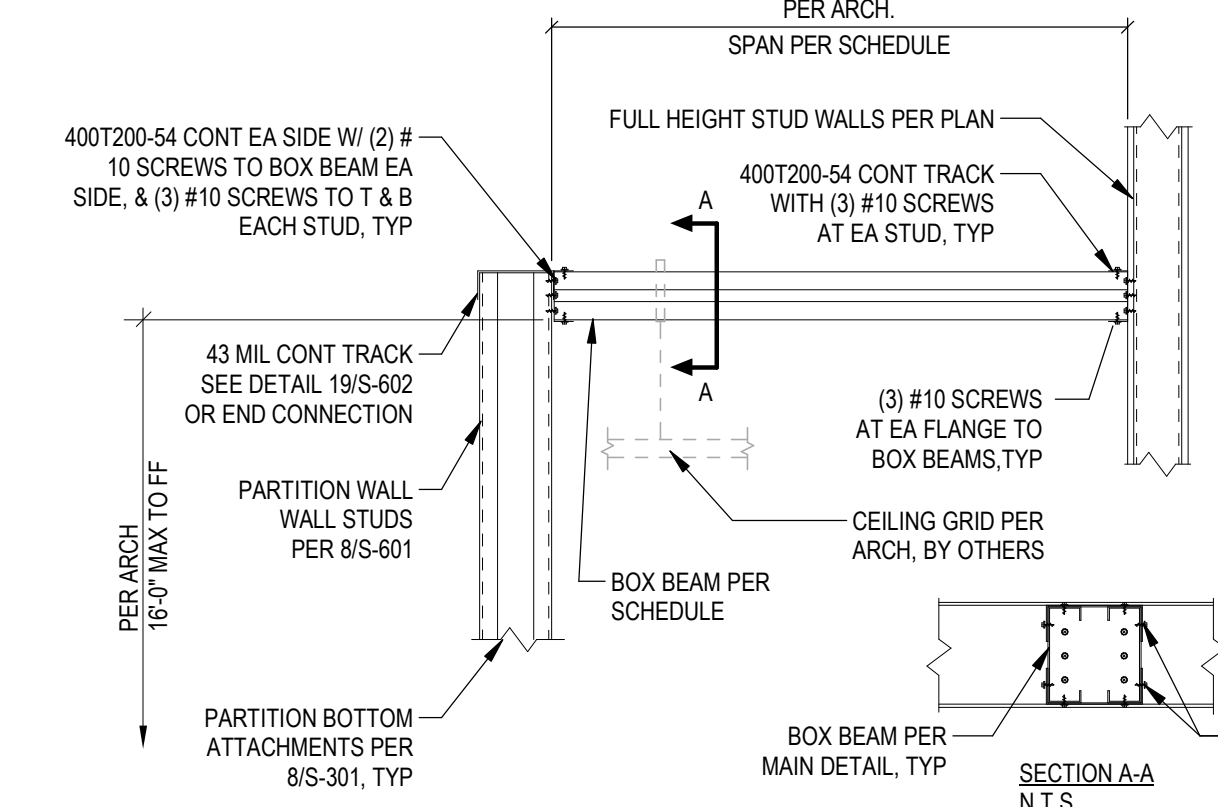


DETAIL A-A'

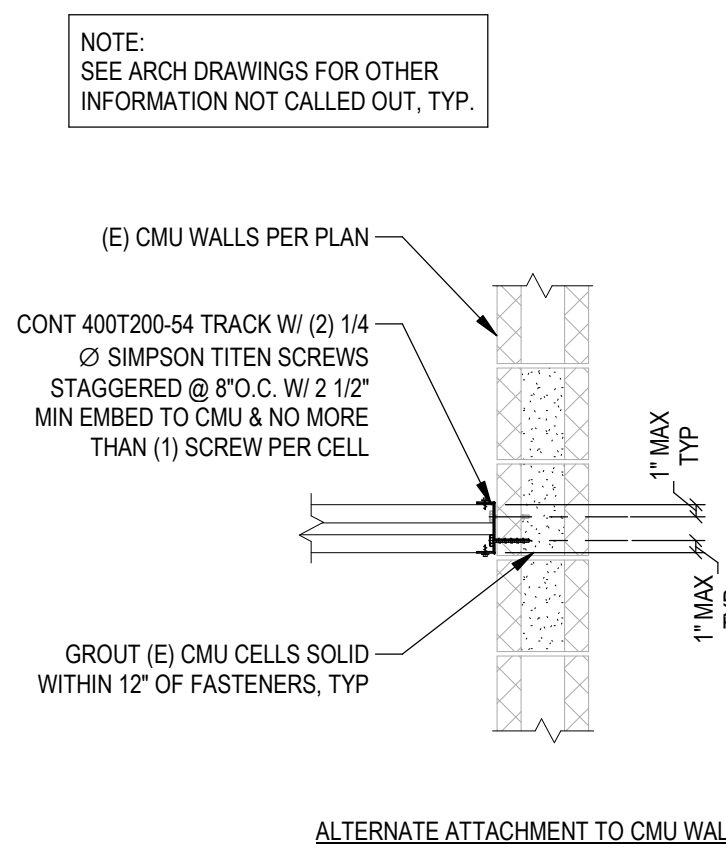


19 STAND-ALONE PARTIAL HEIGHT WALL
S-602 1 1/2" = 1'-0"

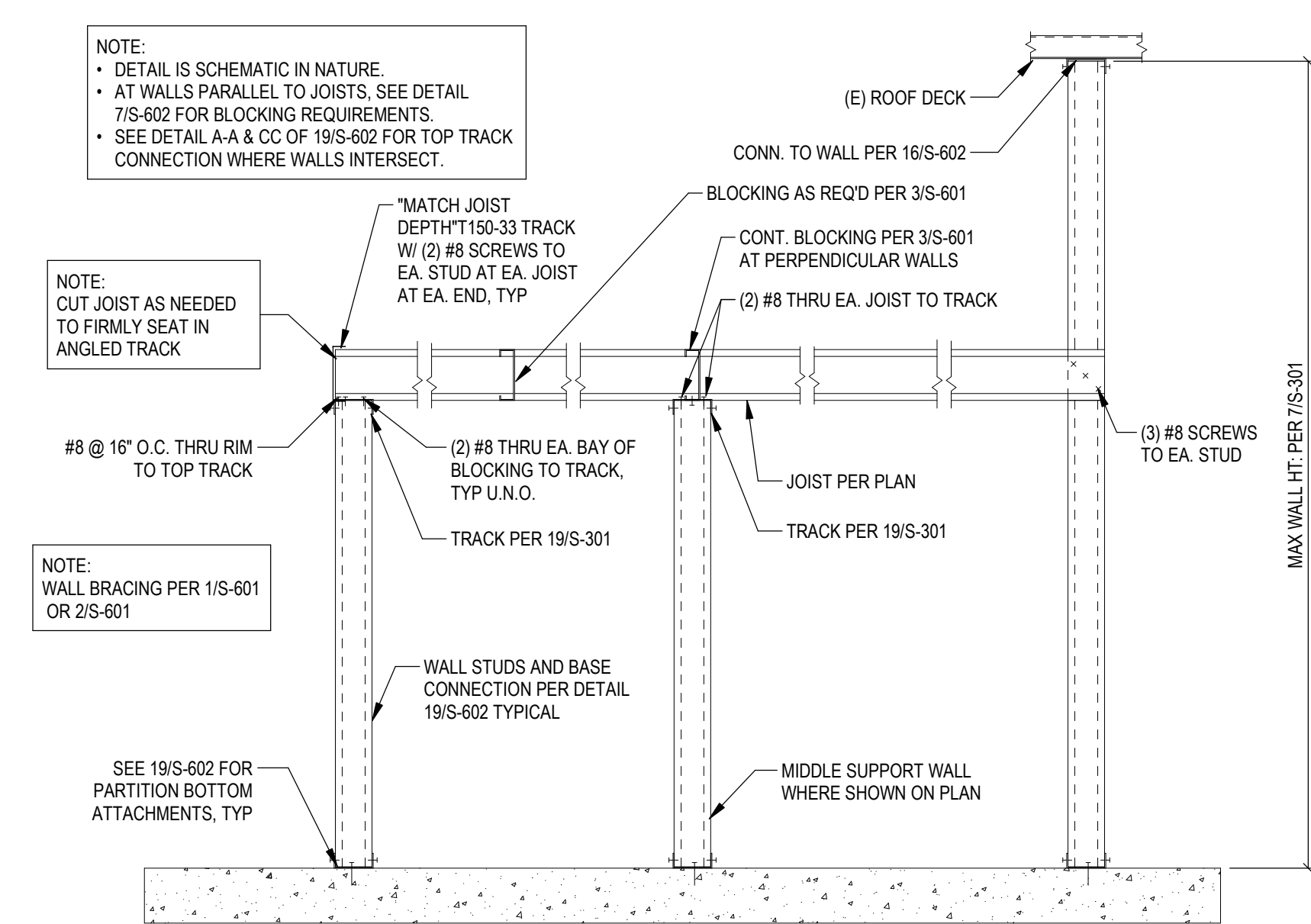
BOX BEAM SCHEDULE		
MAXIMUM SPAN	STUDS	ATTACHMENTS
17 FEET	400S162-43	400T150-43
25 FEET	600S200-54	600T200-54



SECTION A-A

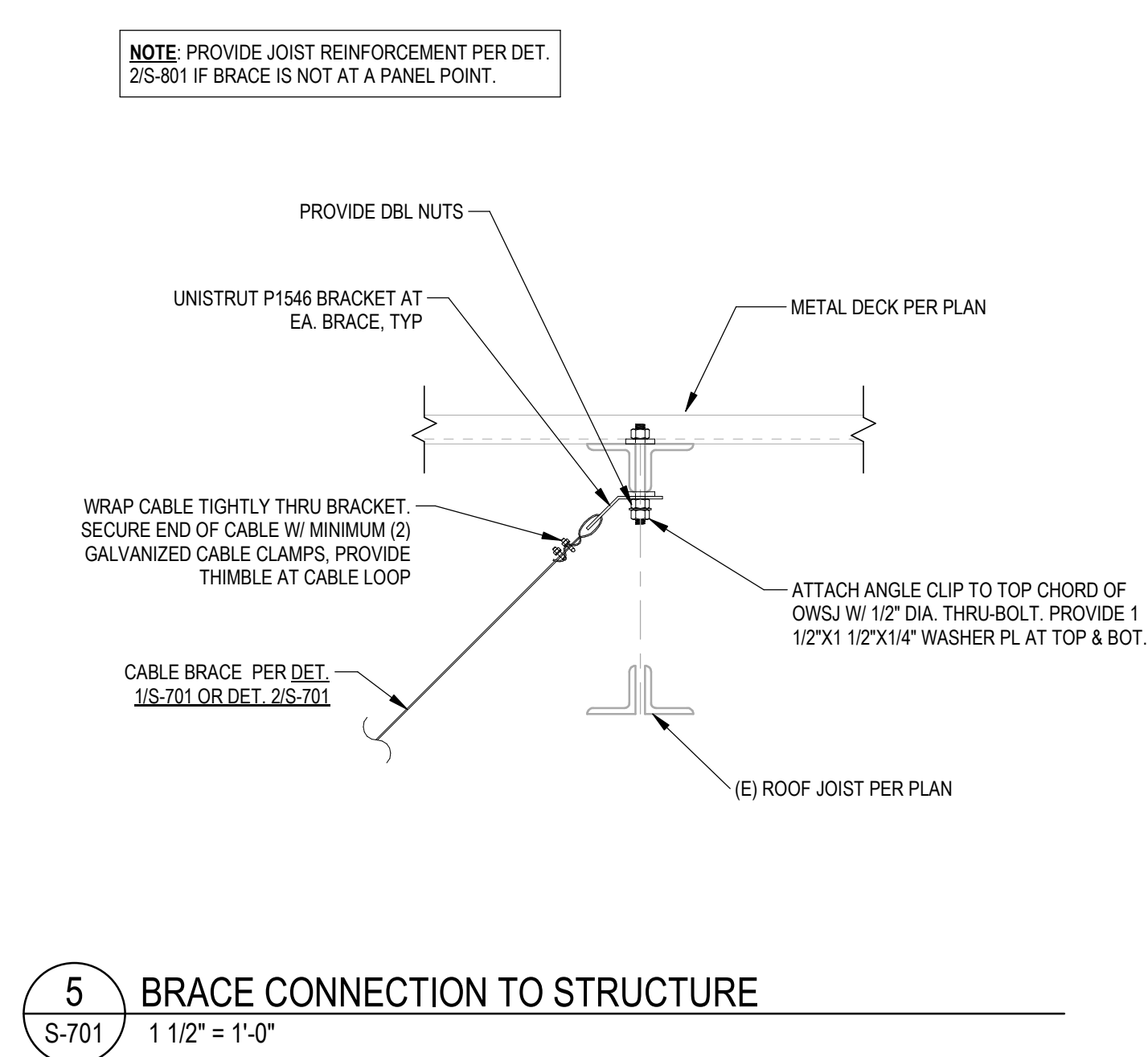
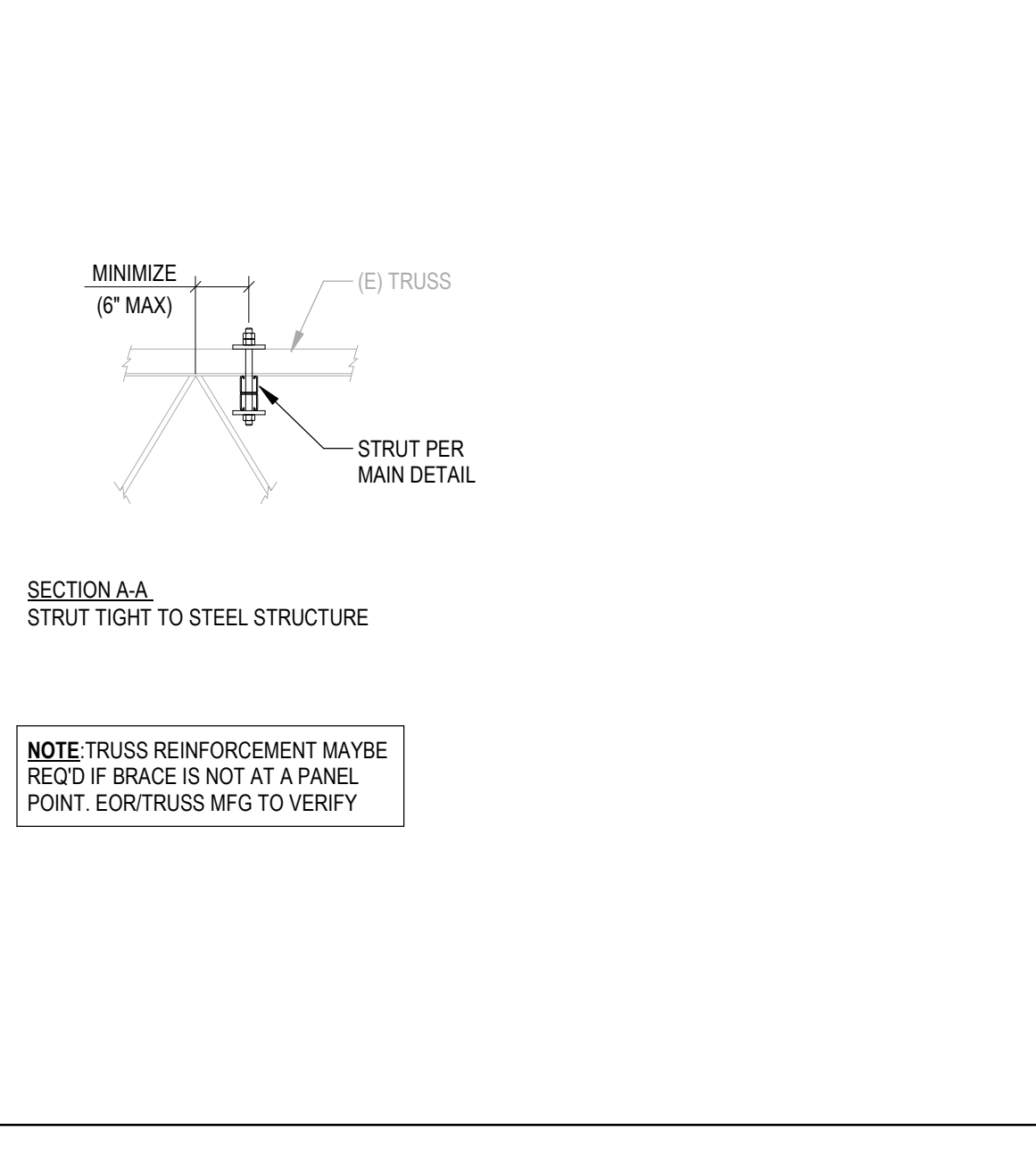
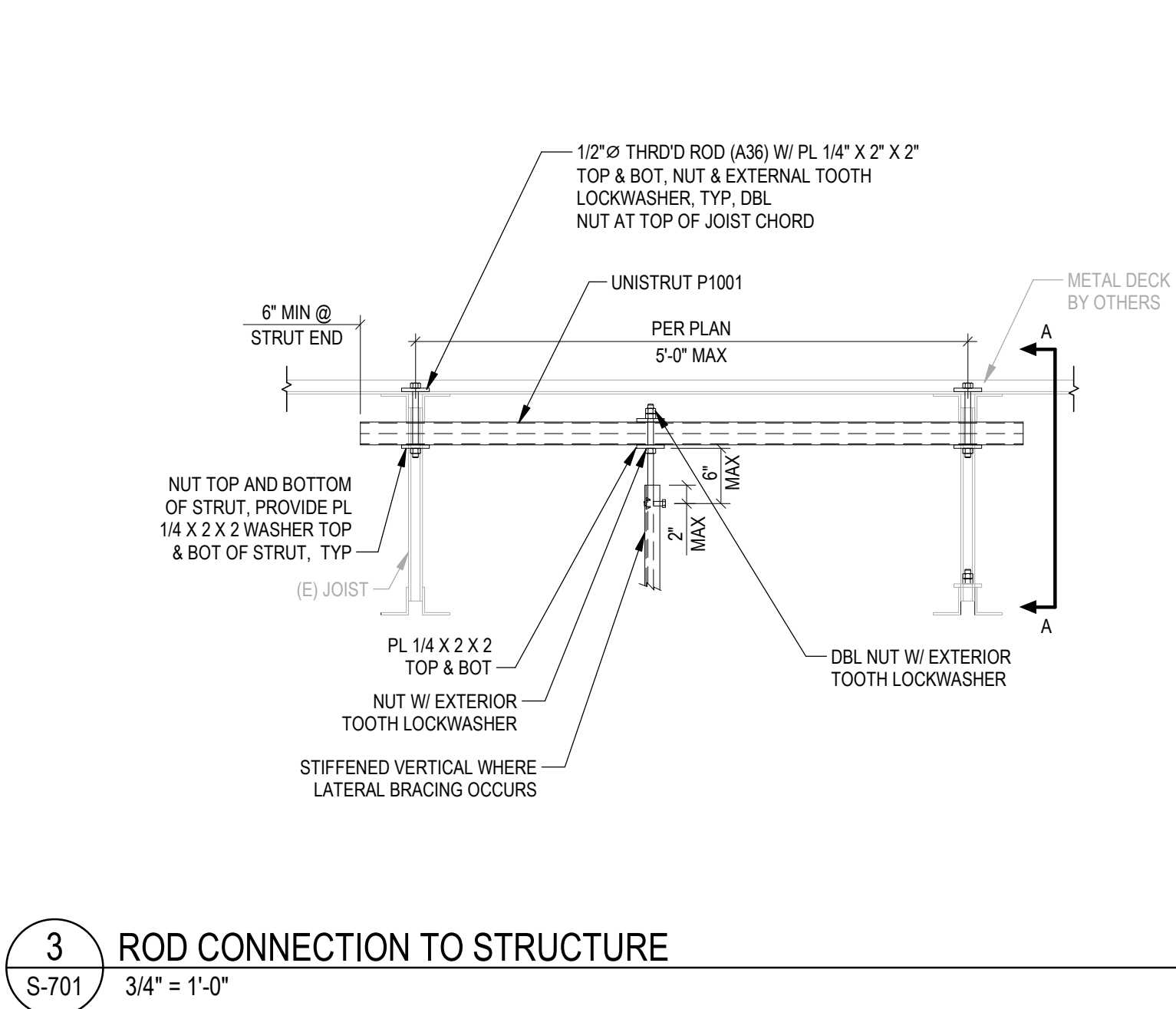
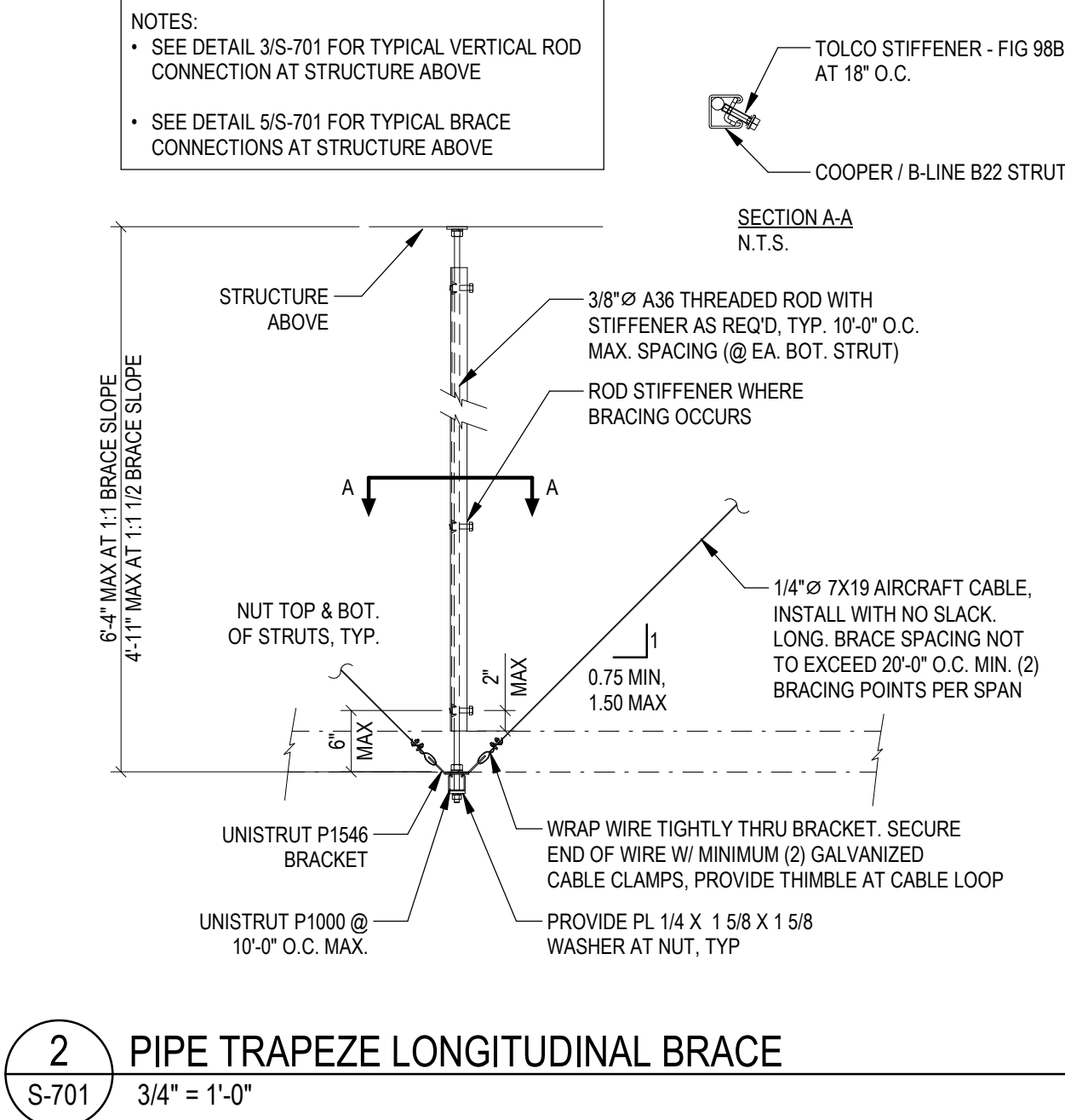
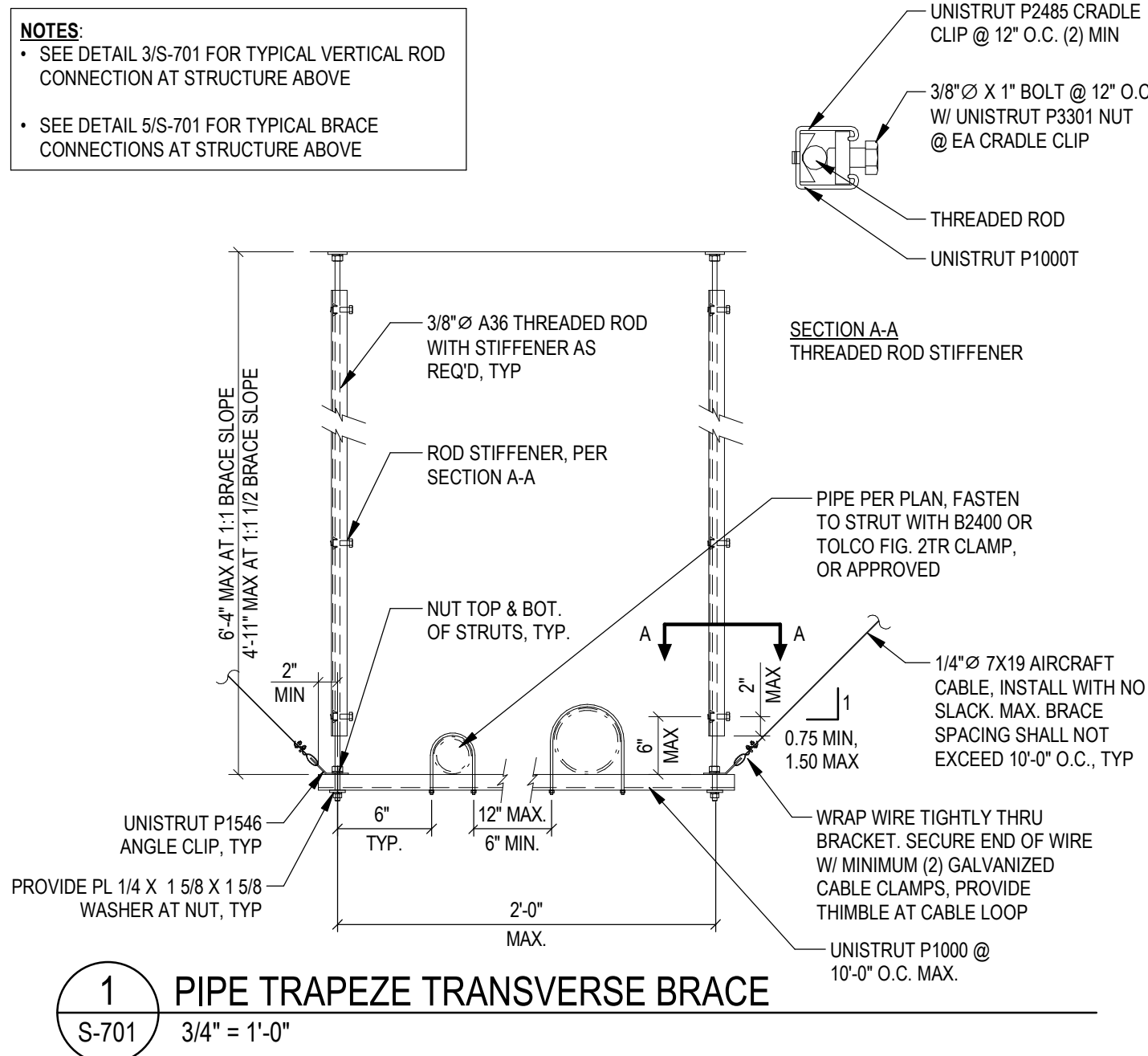


ALTERNATE ATTACHMENT TO CMU WALL



18 STEEL STUD JOIST CONFIGURATION
S-602 3/4" = 1'-0"

16 WALL BRACE TO EXISTING WALL
S-602 3/4" = 1'-0"



CLIENT STORE NUMBER

235

CLIENT INFORMATION

INDIRECT INFORMATION

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CallisonRTKL Inc.
CallisonRTKL Inc.
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2400 Seattle, WA 98101

006-132564.81

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BELLEVUE, WA 98004
425 614-0949 / INFO@AU-ENG.COM
JOB NO: 21173

PROJECT INFORMATION

REI-GLENWOOD SPRINGS

3216 S. GLEN AVENUE
GLENWOOD SPRINGS,
CO. 81601

SIGNATURE/SEAL

DRAWING SOURCE LOG

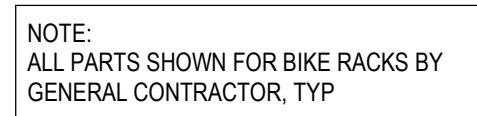
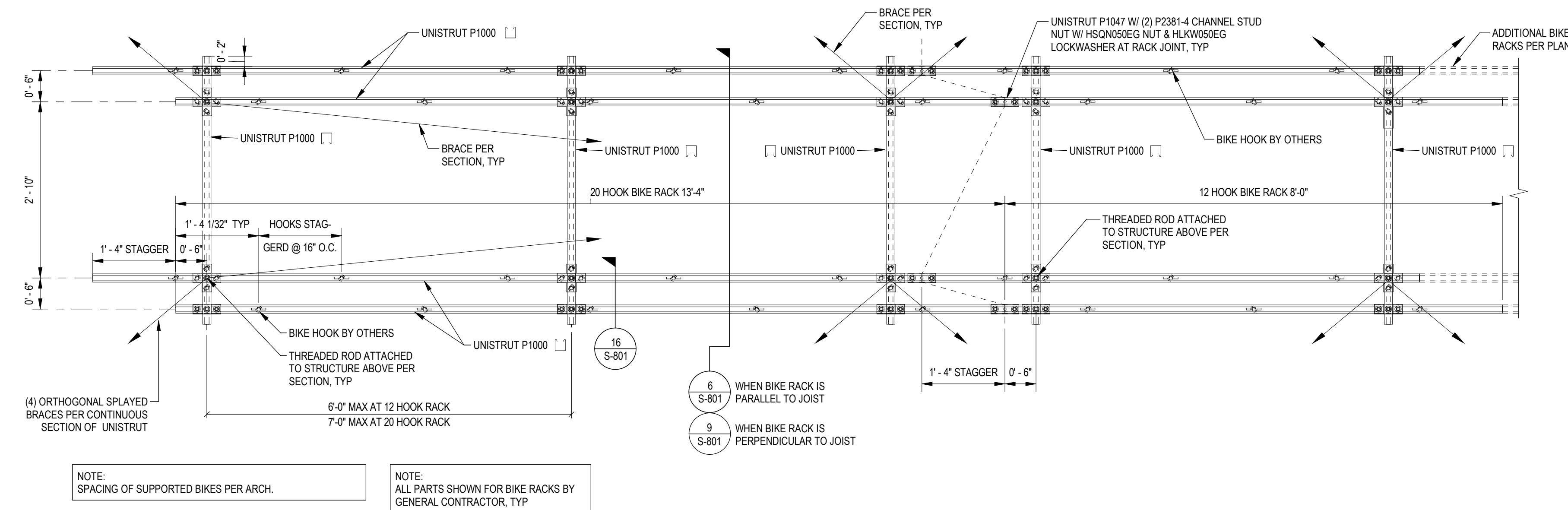
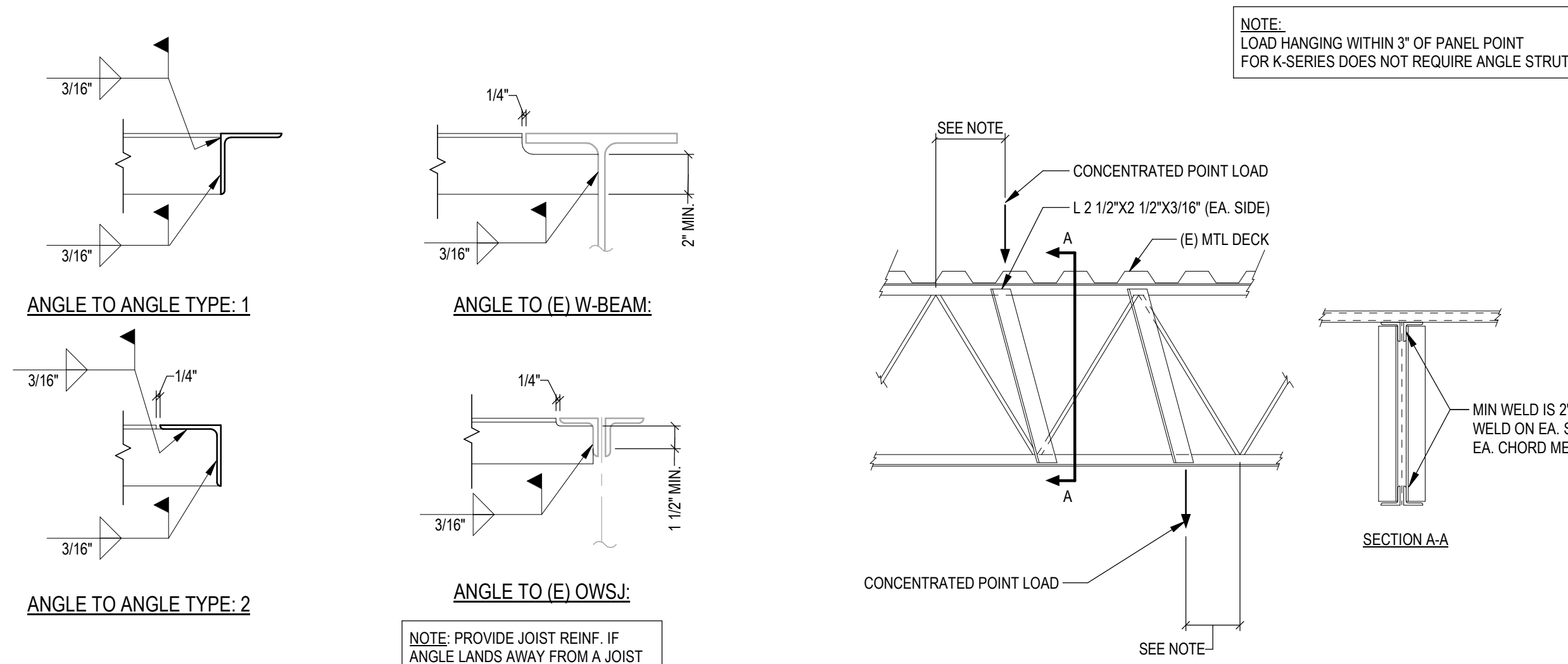
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11/05/21	PERMIT SET	
11/08/21	BID SET	

SHEET TITLE

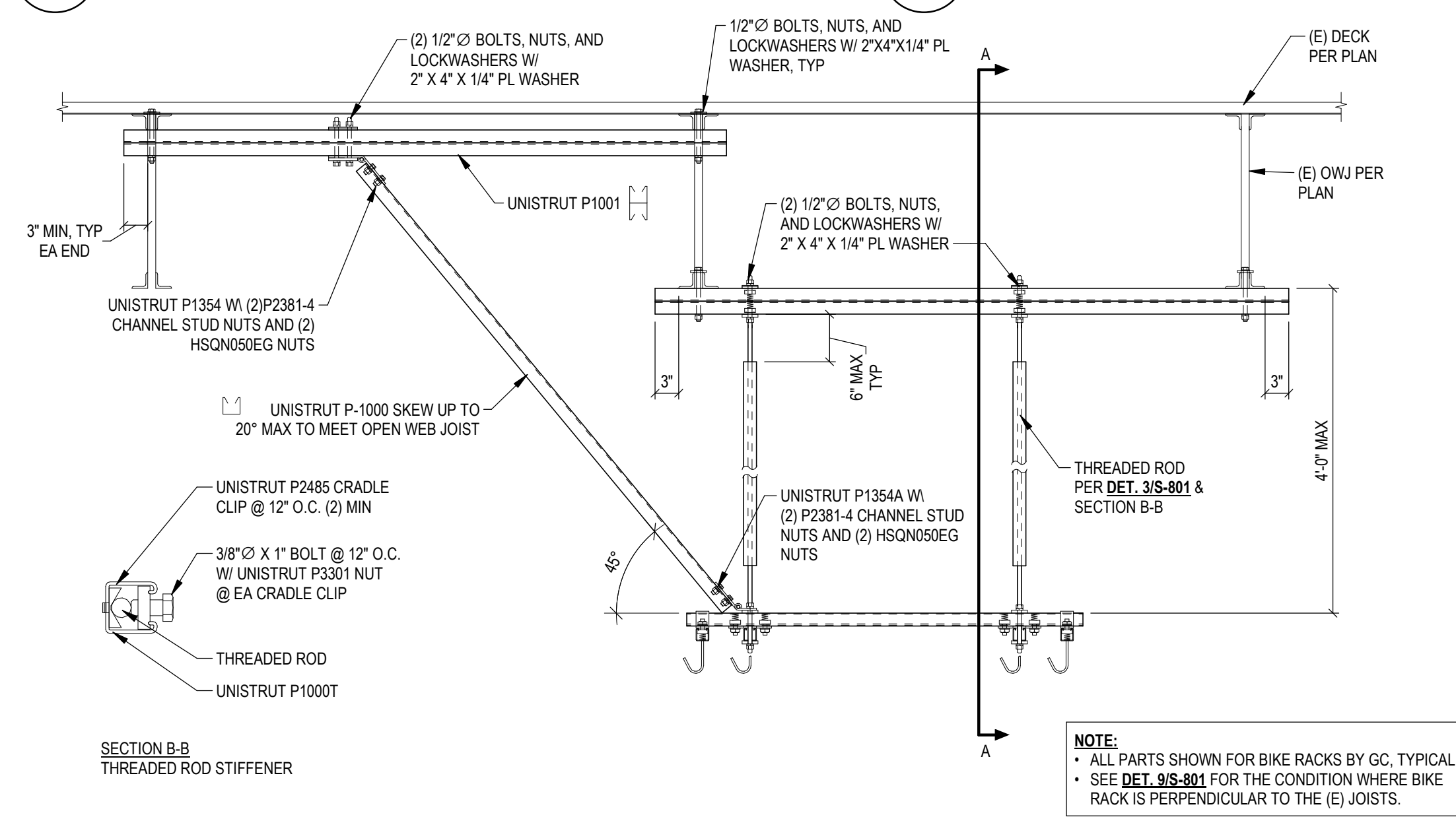
PIPE SUPPORT DETAILS

SHEET NUMBER

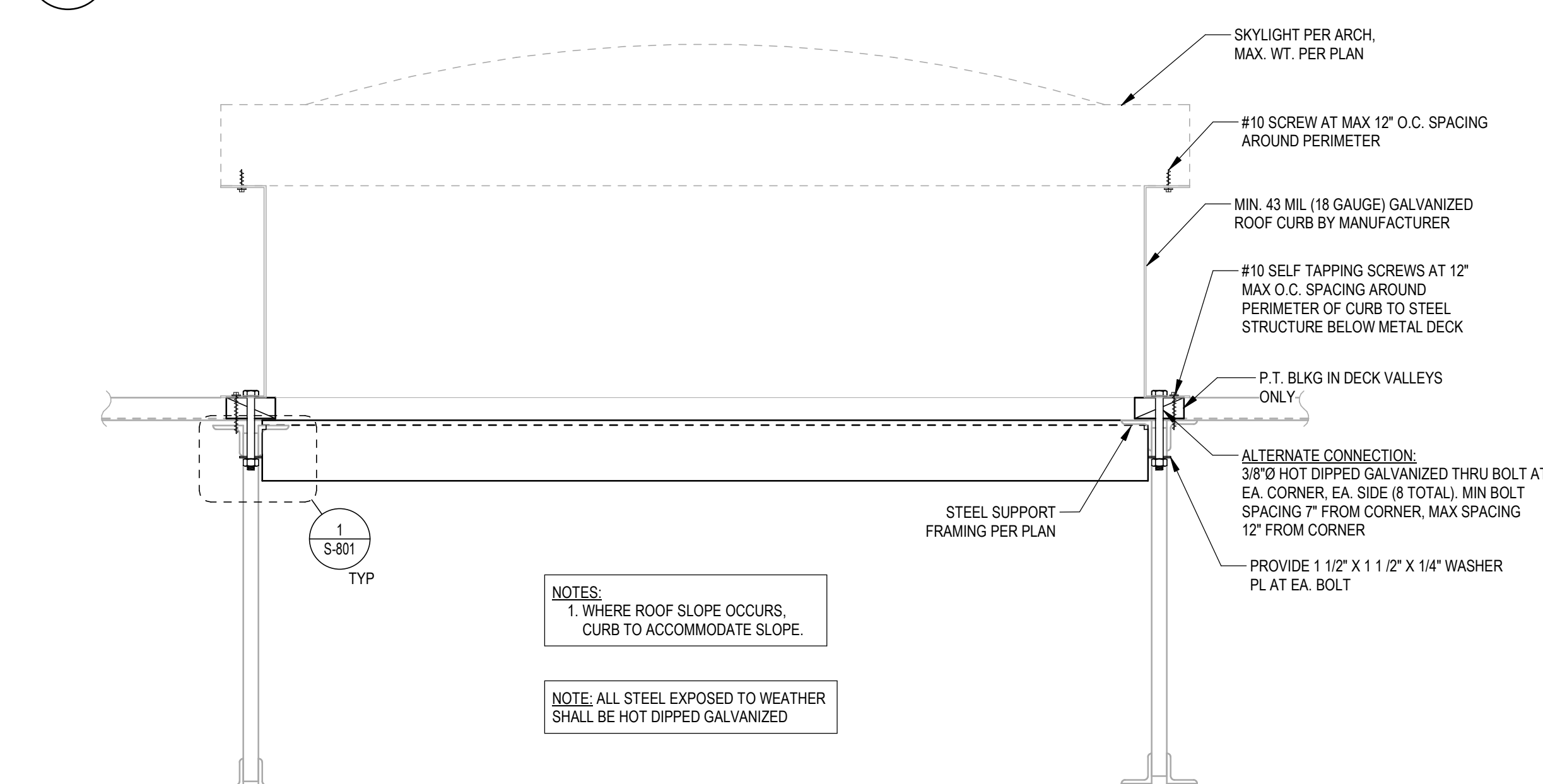
S-701



1 TYPICAL ANGLE CONNECTIONS



6 SUSPENDED BIKE RACK SECTION PARALLEL TO (E) JOIST
S-801 $\frac{3}{4}" = 1'-0"$



11 SKYLIGHT ATTACHMENT
S-801 1 1/2" = 1'-0"